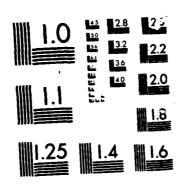
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DESIGN CALCULATIONS 81'- MLW STRUCTURE

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approximately 81 feet of water.

This report (No. 27-771-94) is part of the documentation required by U.S. Government Contract No. N62477-76-C-0179, Modification No. P0001, let by the Naval Facilities Engineering Command, Department of the Navy, Chesapeake Division with Crest Engineering, Inc., Tulsa, Oklahoma.

DESIGN CALCULATIONS
81' MLW PLATFORM



EAST COAST AIR COMBAT MANEUVERING RANGE
OFFSHORE KITTY HAWK, NORTH CAROLINA
CONTRACT NO. N62477-76-C-0179
MODIFICATION NO. PO001

Report No. 27-771-94

Prepared for

NAVY FACILITIES ENGINEERING COMMAND DEPARTMENT OF THE NAVY CHESAPEAKE DIVISION

Ву

CREST ENGINEERING, INC. TULSA, OKLAHOMA

September 1976

TABLE OF CONTENTS:

SECTION		TITL	<u>.E</u>	PAGE NO.
1.0		INTRODUCTION		
	1.1 1.2 1.3 1.4 1.5	Purpose of Document Description of Stru Design Criteria Design Assumptions Design Summary		1.01 1.02 1.07 1.11 1.13
2.0		STRUCTURAL DRAWINGS	S .	
	2.1 2.2 2.3 2.4 2.5	Assembly Drawing		2.01 2.02 2.03 2.04 2.12
3.0		STRUCTURAL IDEALIZA		
	3.1 3.2 3.3 3.4 3.5 3.6 3.7	Sketches - Plans ar	nd Elevations	3.01 3.03 3.21 3.21 3.25 3.26 3.29
4.0	4.1 4.2 4.3 4.4 4.5	BASIC LOADS Introduction Dead Loads Live Loads Wind Loads Wave Loads	Accesion For NTIS CRA&I DTIC TAB Unannou ced Justification	4.01 4.02 4.03 4.04 4.05
			By Diet ibution/ Availability Codes Dist Avail a .d / or Special	Querideo



SECTION		TITLE	PAGE NO.
5.0		> LOADING CONDITIONS :	
	5.1 5.2		5.01 5.02
6.0		SPACE FRAME ANALYSIS	
	6.1 6.2 6.3 6.4 6.5	Joint Deflections and Rotations	6.01 6.02 6.06 6.10 6.19
7.0		TUBULAR JOINT ANALYSIS	
	7.1 7.2 7.3 7.4 7.5	Joint Geometry - Primary Joints Punching Shear Analysis - Primary Joints Joint Geometry - Secondary Joints	7.01 7.03 7.05 7.24 7.28
8.0		PILE-JACKET CONNECTION .	
	8.1 8.2 8.3 8.4 8.5 8.6	Jacket to Shim Shim to Pile Shim Stress	8.01 8.02 8.03 8.03 8.04 8.04
9.0		PILE ANALYSIS	
	9.1 9.2 9.3 9.4 9.5 9.6 9.7	Pile Loads Pile Capacity Curve Pile Driving Resistance Curves	9.01 9.02 9.03 9.04 9.05 9.08 9.11

SECTION		TITLE	PAGE NO.
10.0		, INSTALLATION ANALYSES	
		Lifting Eyes - Jacket Lift Analysis - Jacket	10.01 10.02 10.04 10.09 10.22
11.0		CORROSION PROTECTION	
	11.1 11.2 11.3 11.4 11.5	Design Data Submerged Zone	11.01 11.02 11.03 11.05 11.06
12.0		MATERIAL LIST AND WEIGHT	
	12.1 12.2 12.3 12.4 12.5 12.6	Material Listing and Weight - Superstructure Material Listing and Weight - Jacket Material Listing and Weight - Boat Landing	12.01 12.02 12.05 12.08 12.11 12.13
APPENDIX A		ENGINEERING DATA	
	A.1 A.2	Environmental Data Wave Profiles	
APPENDIX B		COMPUTER OUTPUT	
	B.1 B.2 B.3 B.4 B.5 B.6	SEALOAD - 50 Year Storm STRAN - 50 Year Storm SAPCHK - Primary Joints SAPCHK - Secondary Joints Lift Analysis Material Listing	

SECTION 1.0 INTRODUCTION

1.1 PURPOSE OF REPORT

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The purpose of this report is to provide a document insuring the structural integrity of one of the four marine structures of the U. S. Navy East Coast Air Combat Maneuvering Range, offshore of Kitty Hawk, North Carolina. This structure is identified as Structure 1 at Site 1, resting in approximately 81 feet of water.

This report (No. 27-771-94) is part of the documentation required by U.S. Government Contract No. N62477-76-C-0179, Modification No. P0001, let by the Naval Facilities Engineering Command, Department of the Navy, Chesapeake Division with Crest Engineering, Inc., Tulsa, Oklahoma.

1.2 DESCRIPTION OF STRUCTURE

1.2.A Purpose of Structure

This marine structure, at Site 1, is part of a series of structures comprising the U.S. Navy East Coast Air Combat Maneuvering Range. Their purpose is to provide a platform to support electronic instrumentation necessary for the proper functioning of the East Coast Air Combat Maneuvering Range.

The equipment on this structure includes:

- Navigation and aircraft warning lights attached to the top
 of each of the three columns.
- A signal horn attached to the underside of the Equipment
 Deck extending toward the sea.
- A receiver-transmitter assembly attached to the Equipment
 Deck in the vicinity of the southwest corner.
- 4. A solar panel assembly attached to the cantilevered deck on the south side of the Equipment Deck.
- A battery assembly fastened just north of the solar panel assembly.
- 6. One air-to-ground antenna attached in the center of the Upper Deck.

- 7. One ground-to-ground cylindrical antenna mounted on a vertical guide attached to the southwest column. The antenna is located at Elevation (+) 15'-0".
- 8. A hand-operated, two-ton marine winch located on the Equipment Deck in the Southeast corner.

1.2.B Location

The site for the East Coast Air Combat Maneuvering Range is approximately 26 miles offshore of Kitty Hawk, North Carolina.

Structure 1 will be erected within a half of a mile of coordinates 13, 051, 860 North by 1, 561,523 West (N35 $^{\circ}$ 56 $^{\circ}$ 59 $^{\circ}$, W75 $^{\circ}$ 15 $^{\circ}$ 59 $^{\circ}$) in 81 feet of water.

The structure will be oriented so that the side of the platform with the cantilevered solar panel deck will face due South. This places the boat landing on the northeast side of the structure, and locates the column with the ground-to-ground antenna nearest to shore.

1.2.C Structural Description

The marine structure consists of a three-pile jacket (template) with equilaterally spaced legs through which steel piles are driven into the seabed. The jacket is then secured to the piling by welding shim plates in the annulus between the jacket leg and piling at the top of the jacket legs. A superstructure consisting of an equipment deck and an upper deck is then attached to the piling above the top of the jacket. This tripod structure has the following features:

- Upper Deck elevation is at (+) 75.0 feet above Mean Low Water to provide an adequate envelope for the hoist on the Equipment Deck.
- Equipment Deck elevation is at (+) 60.0 feet above MLW to provide an air gap of 8.0 feet between the deck and the maximum crest of the 50 year storm.
- 3. To avoid any shadowing of the cells, a cantilevered deck is
- provided on the south side of the Equipment Deck to support the Solar Panel Assembly.
- 4. The only diagonal bracing framing the superstructure is between El. (+) 60.0 feet and El. (+) 45.0 feet.
- 5. The equilateral pile spacing at the pile cut-off El. (+) 16.5 feet is 29.0 feet from centerline to centerline.
- 6. The true jacket batter is 6 to 1 for each of the three legs.

- 7. Horizontal bracings for the jacket are located at El. (+)
 12.0, El. (-) 13.0, El. (-) 47.0, and El. (-) 81.0. In
 addition to the perimeter bracings, secondary horizontal
 bracings connecting the mid-points of the perimeter bracings
 are located at each of the above elevations. Diagonal bracing
 connect the levels.
- 8. A boat landing is provided on the northeast side of the structure from El. (+) 9.0 to El. (-) 3.0.

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9. Boat fenders are attached to the two jacket legs on the boat landing side of the structure to protect the structure from sustaining damage from large impacts of approaching boats. The fenders consist of rubber tires installed around a vertical concrete filled pipe from El. (+) 12.0 to El. (-) 7.5.

1.3 DESIGN CRITERIA

1.3.A Purpose of Structure

1. Wave Data - 50 year storm

MLW Depth	81.0 ft.
Storm Wave Height	60.3 ft.
Storm Wave Period	13.6 sec.
Maximum Storm Tide	4.0 ft.
Maximum Astronomical Tide	4.5 ft.
Extreme Surface Current	4.3 ft./sec.
Mudline Current	0.8 ft./sec.

2. Wind Data

Maximum Gust	174.0 mph
1 Minute Wind	145.0 mph
1 Hour Wind	114.0 mph

The approach of the storm wind and wave can be from any direction.

1.3.B Foundation Criteria

 The basis for the foundation design is a McClelland report to Cubic Corporation entitled "Foundation Investigation East Coast ACMR Ocean Structures, Volume I". The soil information to be used in this analysis is one boring at Site 1 in the aforementioned report. 2. Due to the nature of the sea bottom and sea bottom currents, scouring of 5 feet below the mudline will be used in the piling design to develop the theorectical soil resistance to laterally applied loads.

1.3.C Live Loads

The design live loads will be:

Equipment Deck

150 psf

Top Deck

100 psf

The loads will be distributed uniformly over the entire deck areas.

1.3.D Material

All structural shapes or fabricated tubular goods are to be ASTM A-36 or equal except for the material used for the structure legs at the joint cans which is to be ASTM A-633, Grade A.

1.3.E Corrosion Protection

- All portions of the platform above elevation (-) 4.0 feet will be painted.
- 2. All main structural members located within the splash zone will have an extra 1/2" of sacrificial steel added to their wall thickness. This can be in the form of extra wall thickness or a 1/2" steel plate wrap.

3. The portion of the platform below elevation (-) 4.0 feet will be protected by cathodic protection. This will be provided by sacrificial anodes having a theoretically expected life of twenty years.

1.3.F Pile-Jacket Connection

A THE RESIDENCE OF THE PROPERTY OF THE PROPERT

The platform is analyzed as if the annulus between the jacket and the piling is not grouted. Shim plates will be provided at each horizontal bracing level. Jacket to pile connection is made by welding at elevation (+) 16.5 feet.

1.3.G Design Standards

The criteria employed for determination of structural acceptability are specified by the following documents:

- 1. American Petroleum Institute (API):
 - RP 2A Recommended Practice for Planning,

 Designing and Constructing Fixed Offshore

 Platforms; 7th Edition, January 1976.
 - Spec. 2H Carbon Manganese Steel for Offshore

 Platform Tubular Joints; 1st Edition,

 January 1974; Supplement 1, January 1975.
- American Institute of Steel Construction (AISC):
 Specification for the Design, Fabrication
 and Erection of Structural Steel for Buildings; February 12, 1969.

American Society for Testing and Materials (ASTM):

A36-75 Structural Steel

A633-75 Normalized High-Strength Low-Alloy Structural Steel.

4. American Welding Society (AWS):

D1.1-75 Structural Welding Code. (Rev. 1-76)

1.4 DESIGN ASSUMPTIONS

1.4.A Environmental Criteria

1. Wave Data

(a) Wave Coefficients

$$C_D = 0.74$$
 $C_M = 1.34$

These wave coefficients are the wave coefficients used to generate Dean's Stream Function wave grid profile for a 3.0 ft. diameter pile. It is assumed that these wave coefficients are applicable to all tubular shapes in this structure.

(b) Wave-Current Coupling

The pressures indicated by Dean's Stream Function wave grid profile include the coupling of the wave forces with the current forces.

2. Wind Data

The structure is designed for the one minute wind superimposed on the 50 year storm wave.

1.4.B Equipment Loads

All equipment loads are included in the area live load.

This is a valid assumption because no piece of equipment has

a density to produce a load greater than 150 psf.

1.4.C Marine Growth

- A 1.0" marine growth allowance on the radius is included on all primary jacket members from (+) 0.0 ft. to the mudline.
- The effective diameter for the drag area produced by the marine growth is:

Deff = (Dact + 2.0")
$$(\frac{1.02}{0.74})$$

where 1.02 = C_d for medium barnacle fouling 0.74 = Assumed magnitude of Dean's C_d

1.5 DESIGN SUMMARY

1.5.A Environmental Forces:

Total wind and wave shear force (Maximum-Load Condition #1)	1,270 kips
Total overturning moment	102.454 ftkips

1.5.B Pile Axial Loads:

Maximum Compressive Load (Load Cond. #7 + Pile Weight below Mudline)	2,409 kips
Maximum Tensile Load (Load Cond. #8 + Live Load- Pile Weight below Mudline)	1,746 kips

1.5.C Structural Dimensions:

Piling

Outside Diameter	42 in.
Maximum Wall Thickness	2.00 in.
Minimum Wall Thickness	1.50 in.
Penetration Below Mudline	211 ft.
Jacket	

Spacing at mudifile	37 -1 3/4
Spacing at Work-Point Level	29.0 ft.
Height (Mudline to Work-Point)	97.5 ft.

Superstructure

Equipment Deck Area 556.0 ft.²
Top Deck Area 364.0 ft.²

1.5.D Structural Steel Weight

<u>Item</u>	Structure 1
Piling	772 kips
Superstructure	132 kips
Jacket	296 kips
Boat Landing	24 kips
Boat Fenders	14 kips
Anodes	11 kips
Total	1,249 kips

SECTION 2.0
STRUCTURAL DRAWINGS

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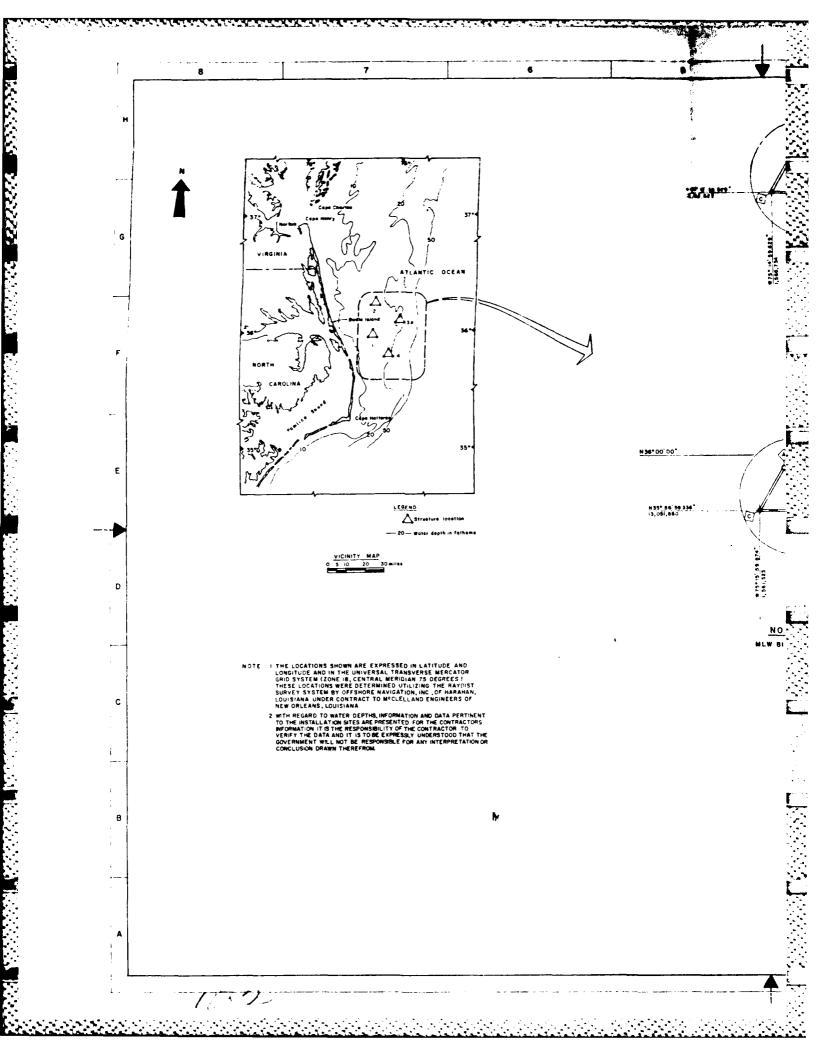
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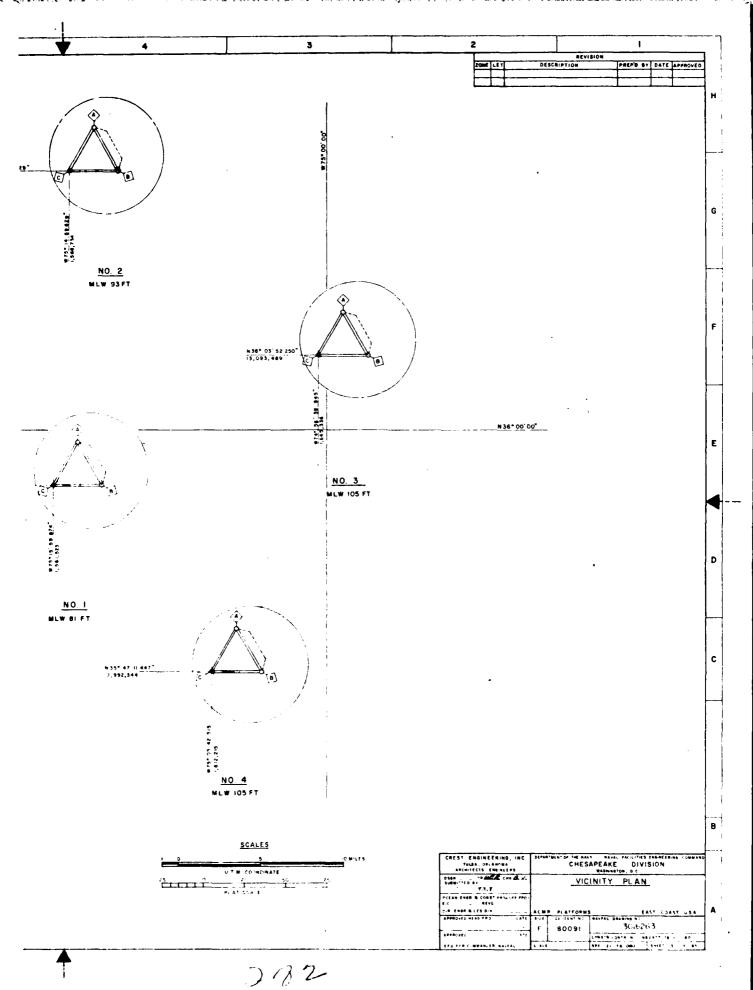
2.1 INTRODUCTION

A few selected structural engineering drawings are included in this chapter for reference to the design calculations. The Introduction to each section in this report lists the appropriate drawings pertinent to that particular section. Reference then can be made to this section of the report for a reduced copy of the drawing.

The drawings comprising this section are as listed below:

3016263	Vicinity Plan	2.02
3016264	Assembly Drawing - 81 Ft. MLW Platform	2.03
3016265	Jacket - Elevations	2.04
3016266	Jacket - Plan at El. (+) 12'-0"	2.05
3016267	Jacket - Plan at El. (-)13'-0" & (-) 47'-0"	2.06
3016268	Jacket - Plan at El. (-) 81'-0"	2.07
3016270 3016272 3016273	Jacket - Pile Shims & Leg Connection Jacket - Lift Eye Details Jacket - Anode Details	2.08 2.09 2.10
3016277	Jacket - Pile Details, Platform 1	2.11
3016278	Superstructure - Elevations	2.12
3016279	Superstructure - Upper Deck Framing	2.13
3016280	Superstructure - Equipment Deck Framing	2.14



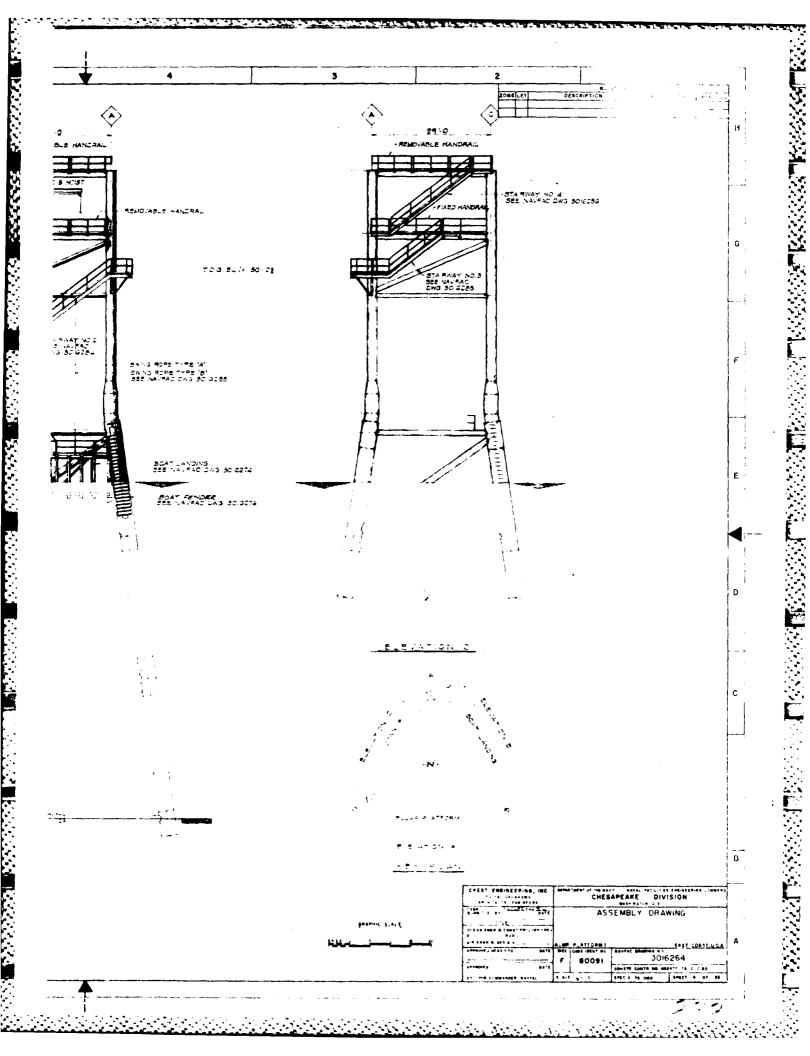


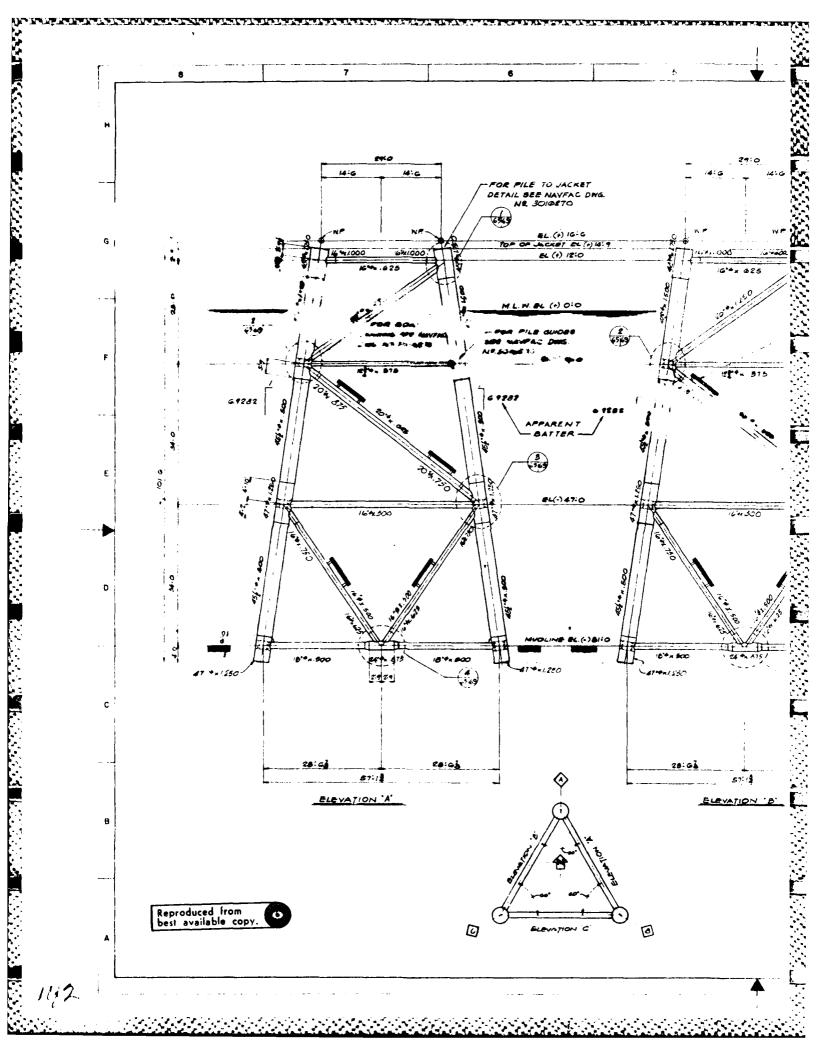
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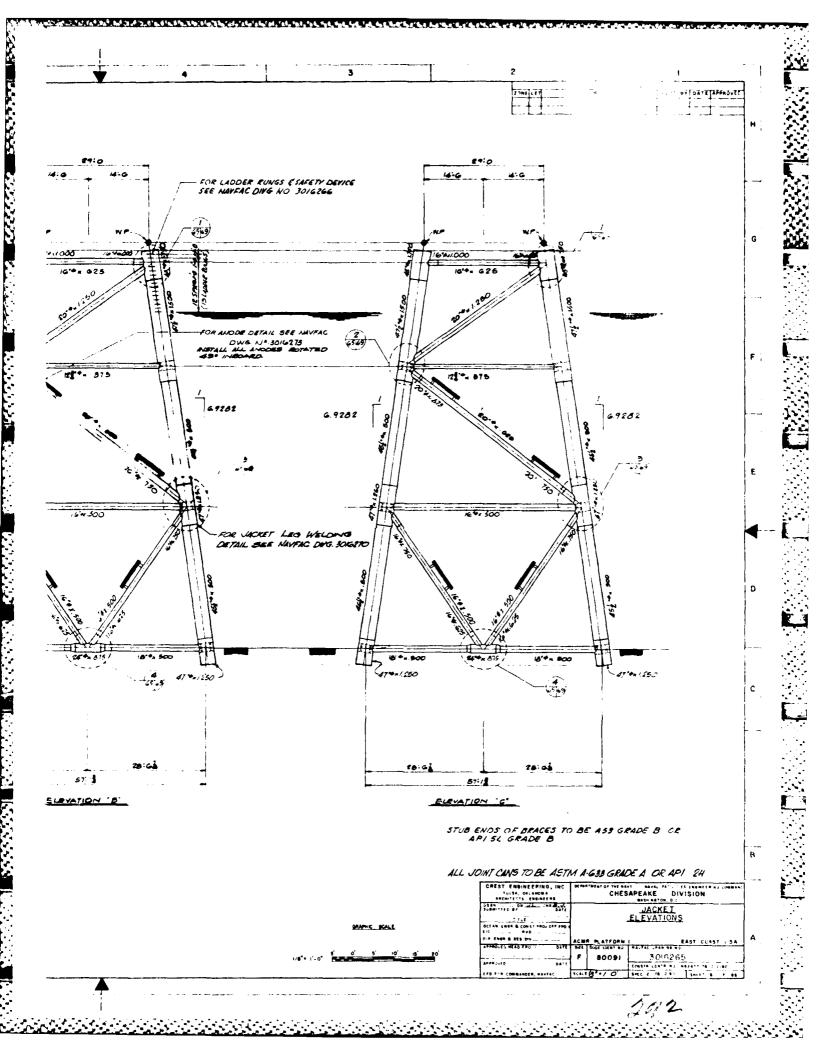
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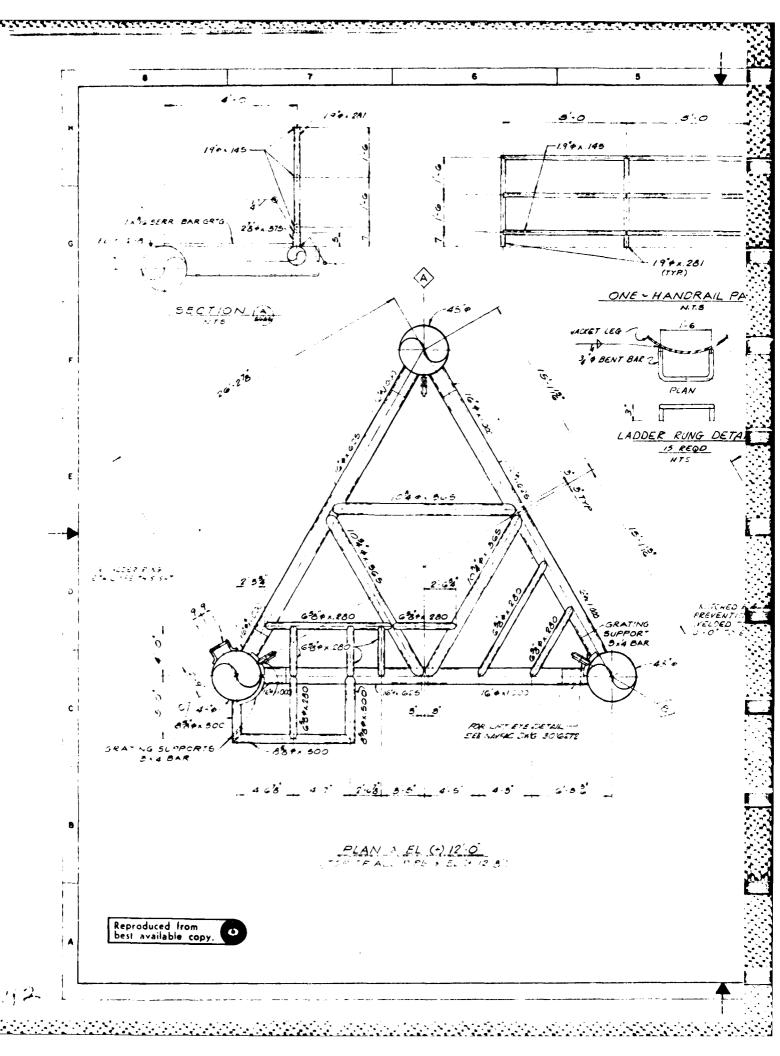
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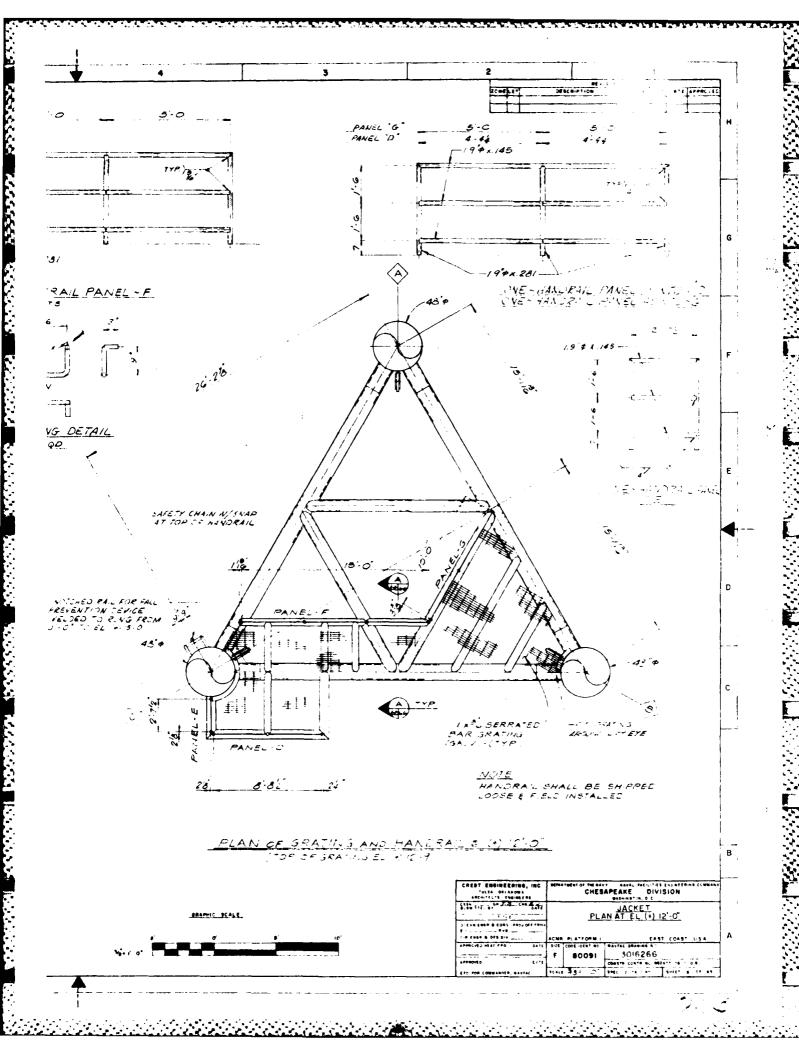
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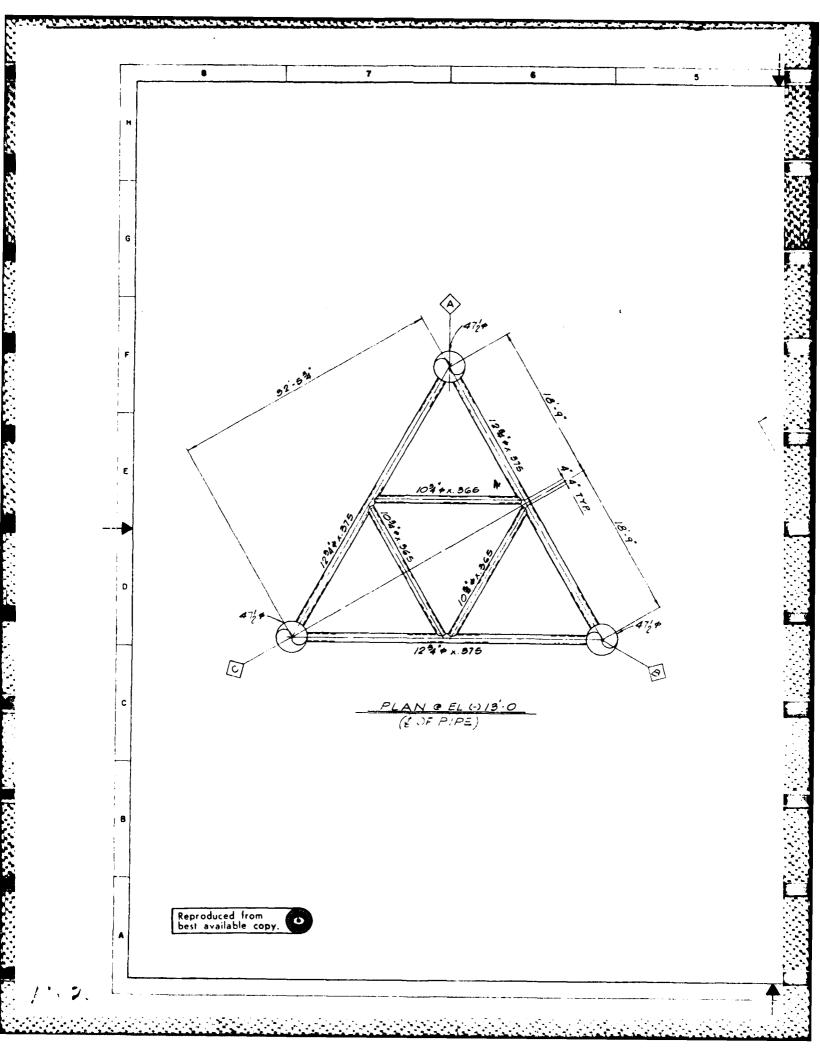


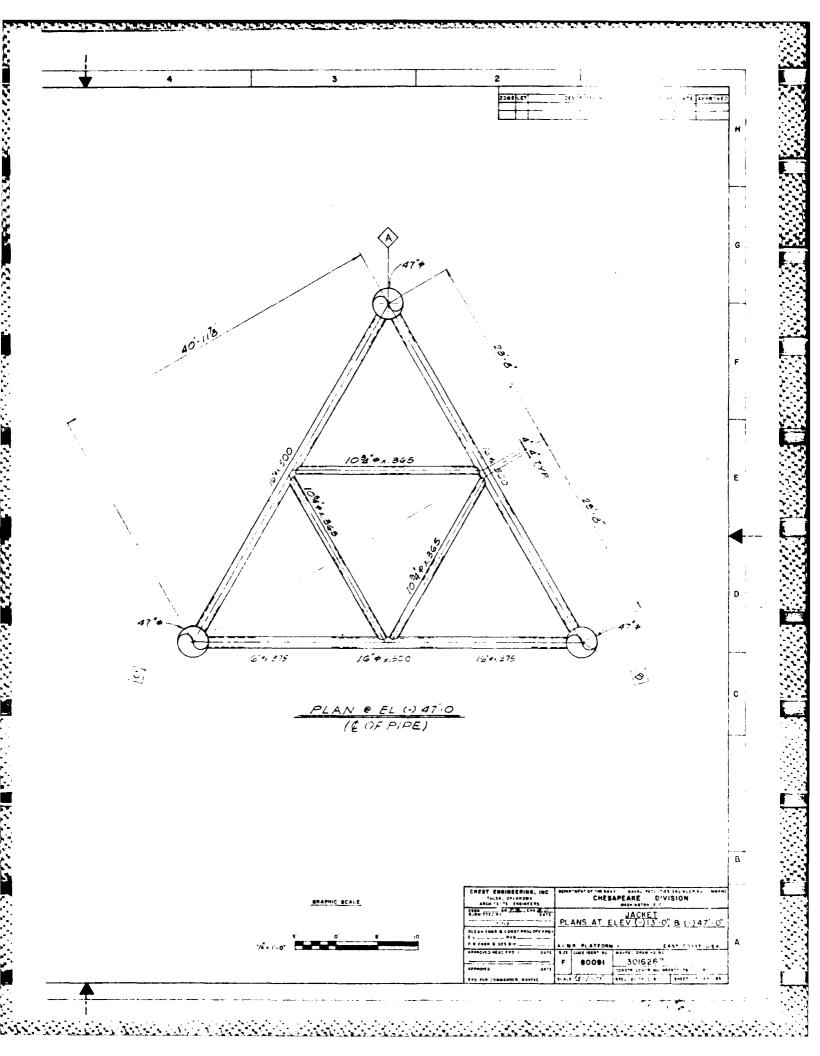






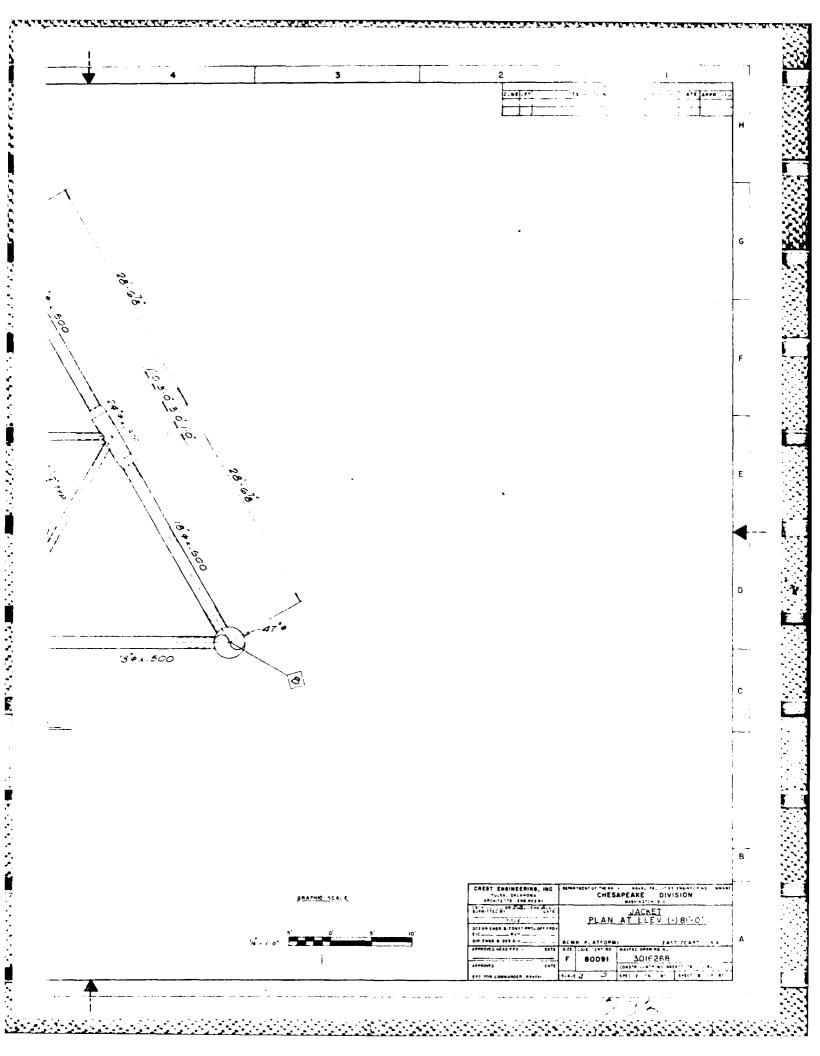


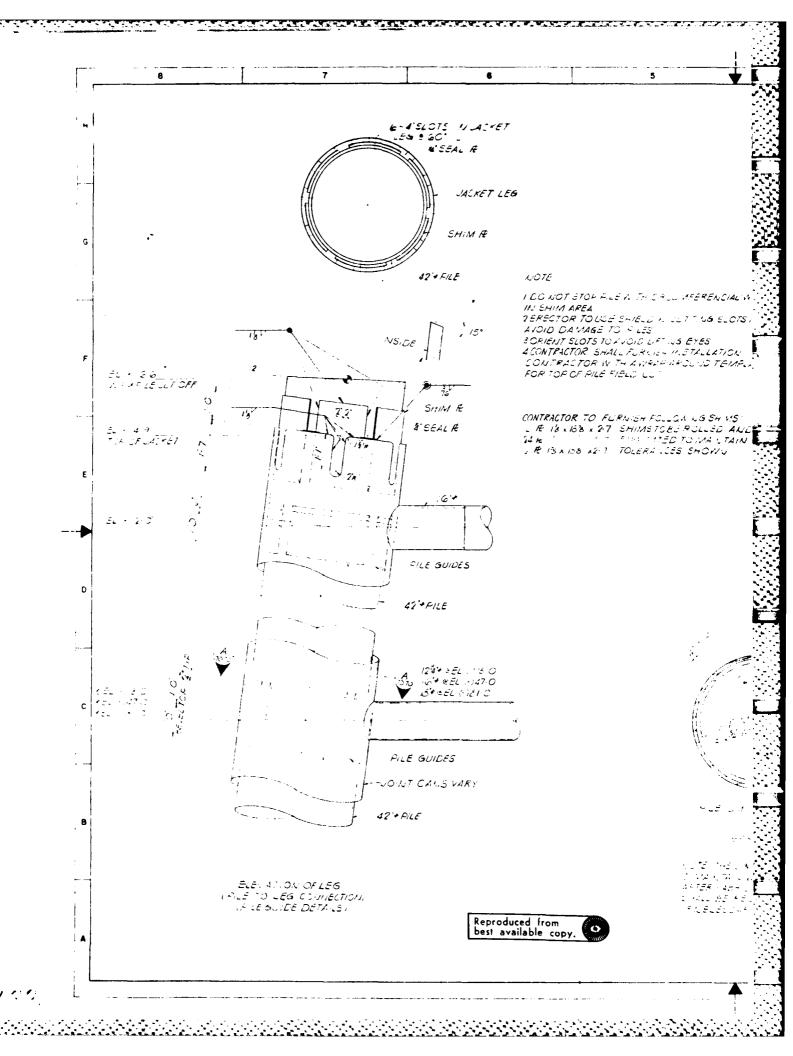


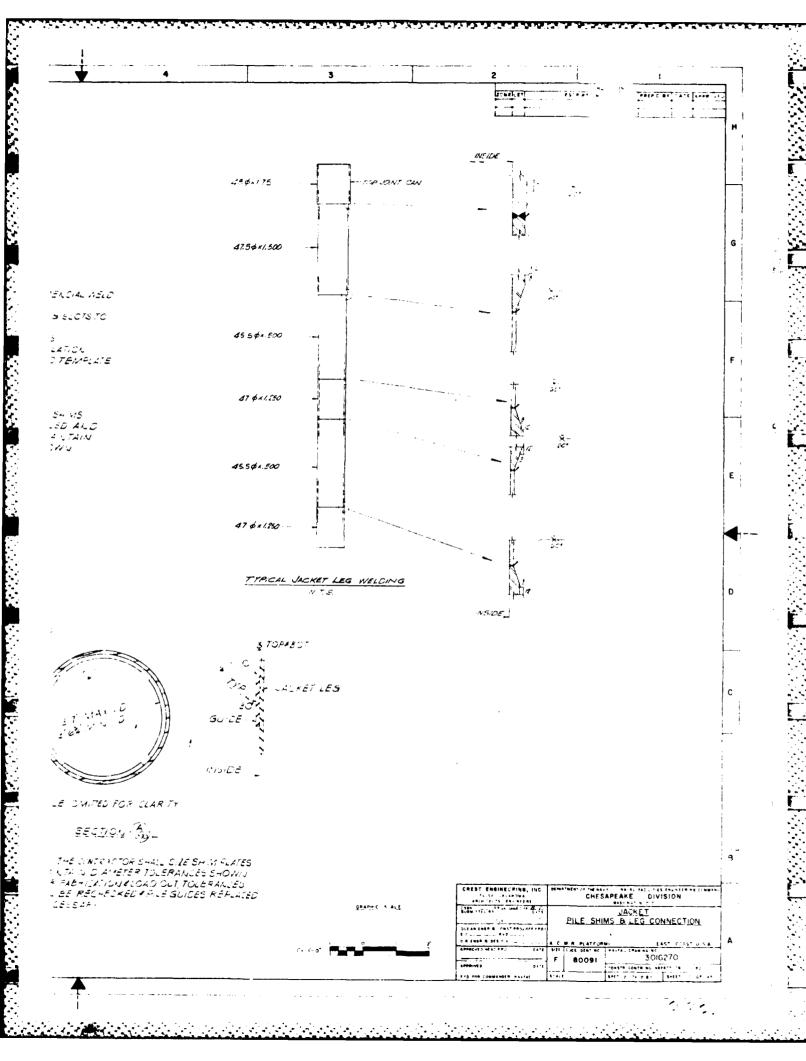


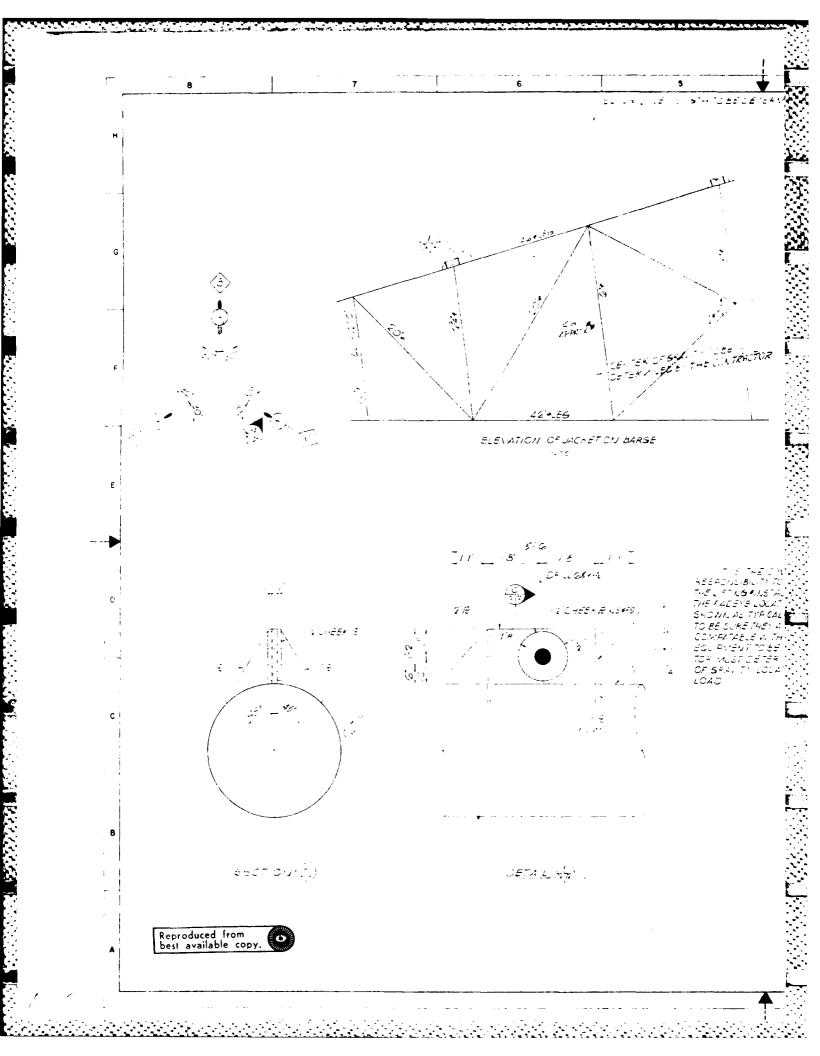
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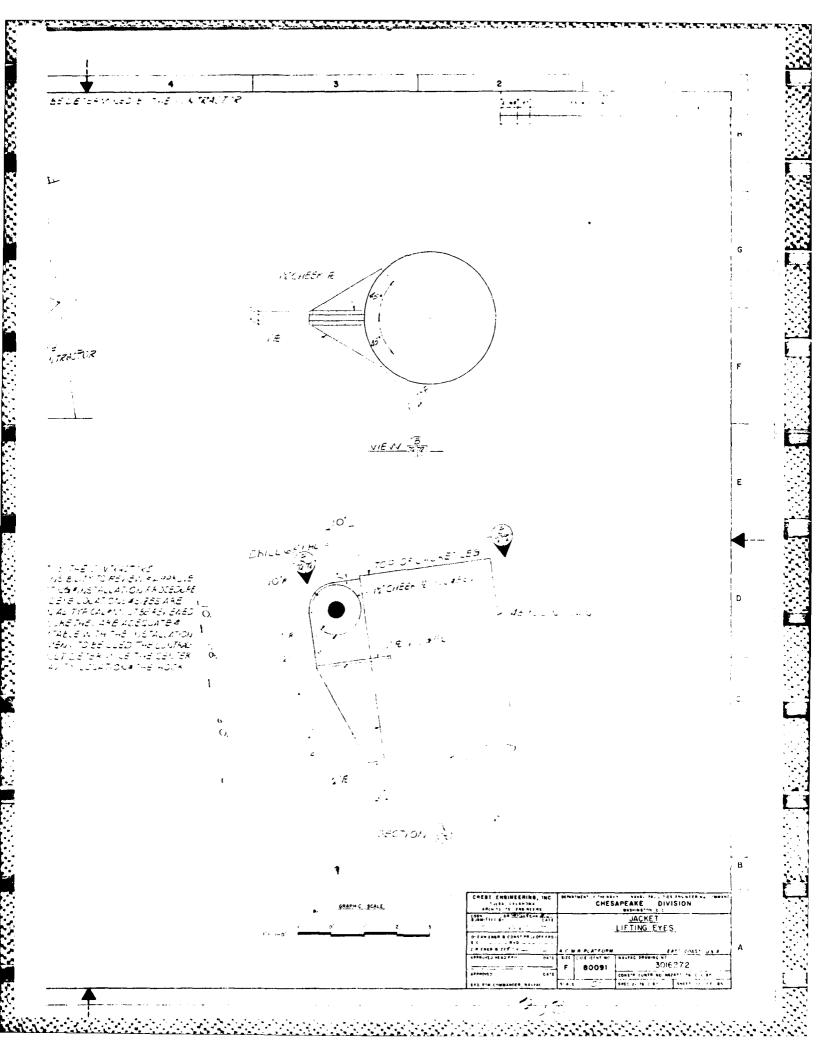
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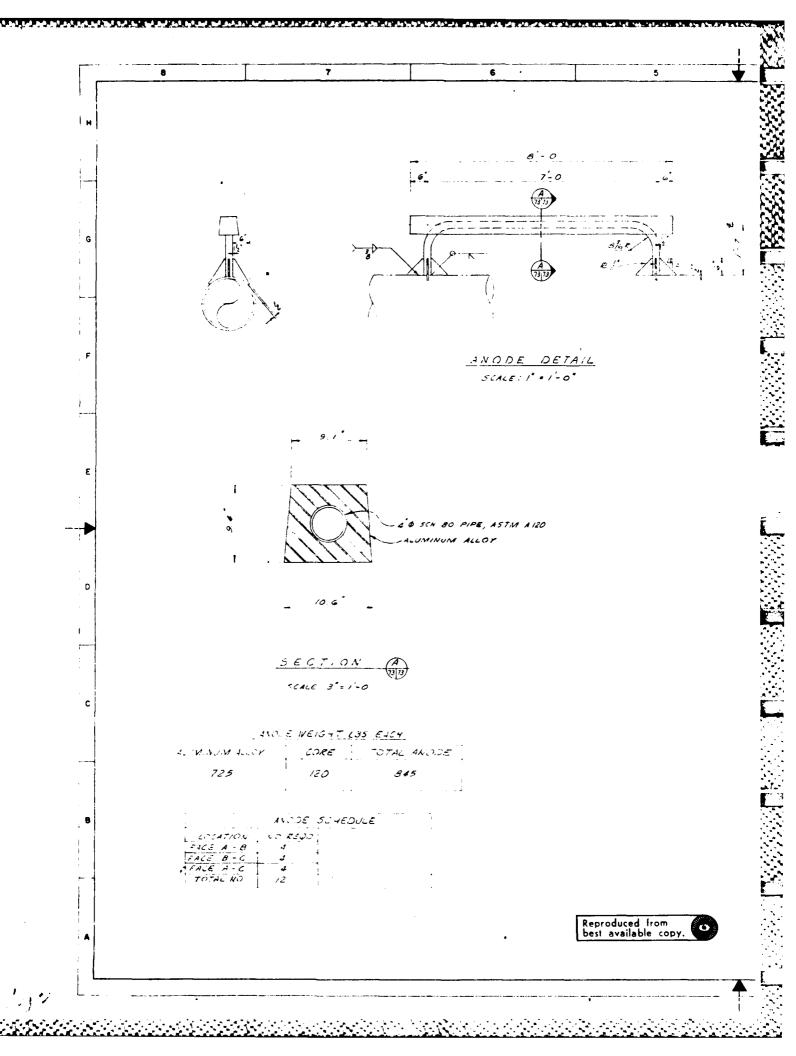


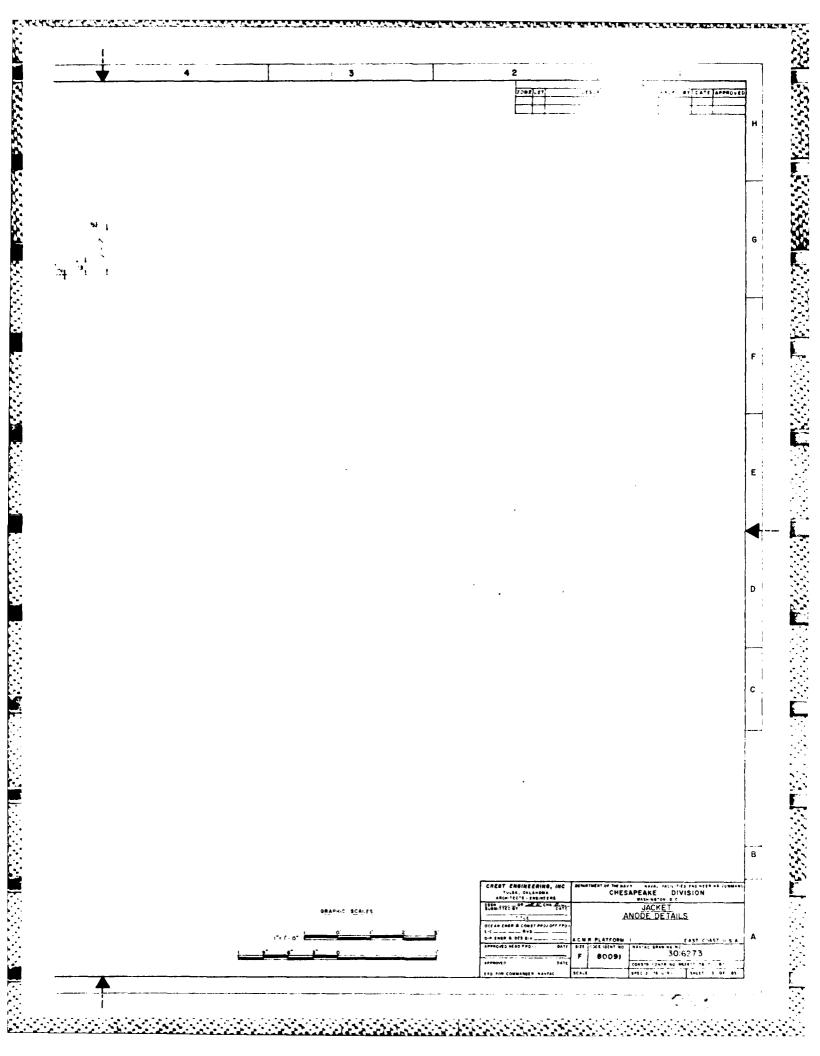


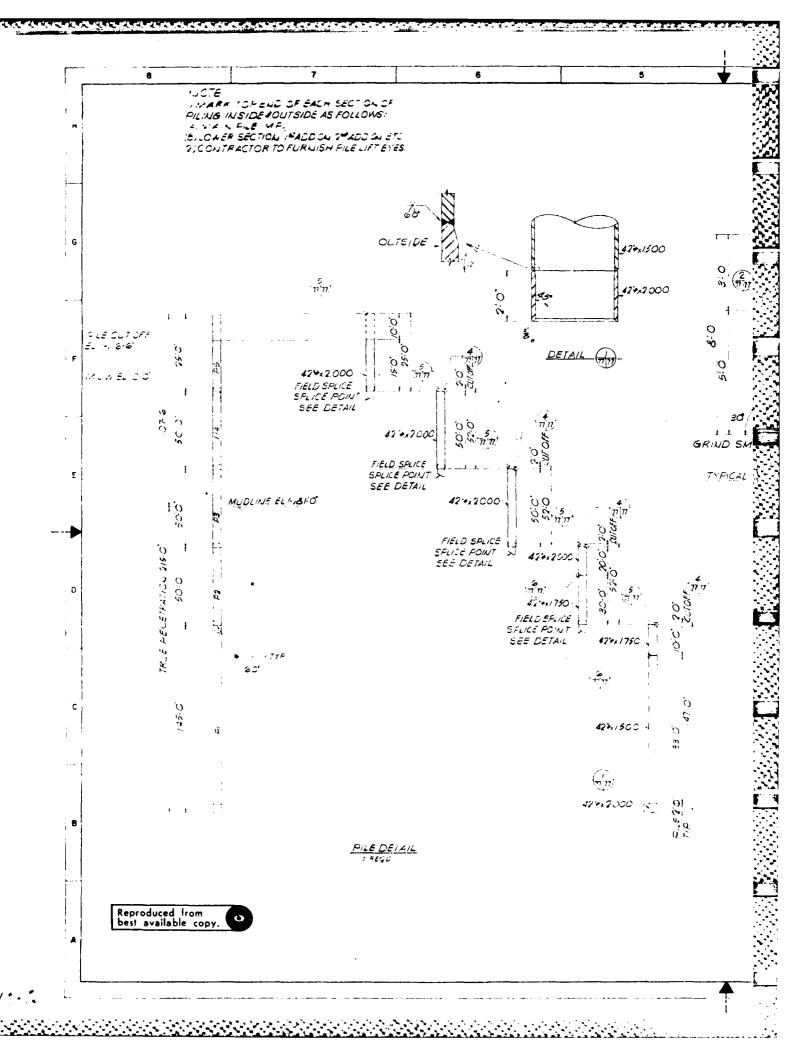


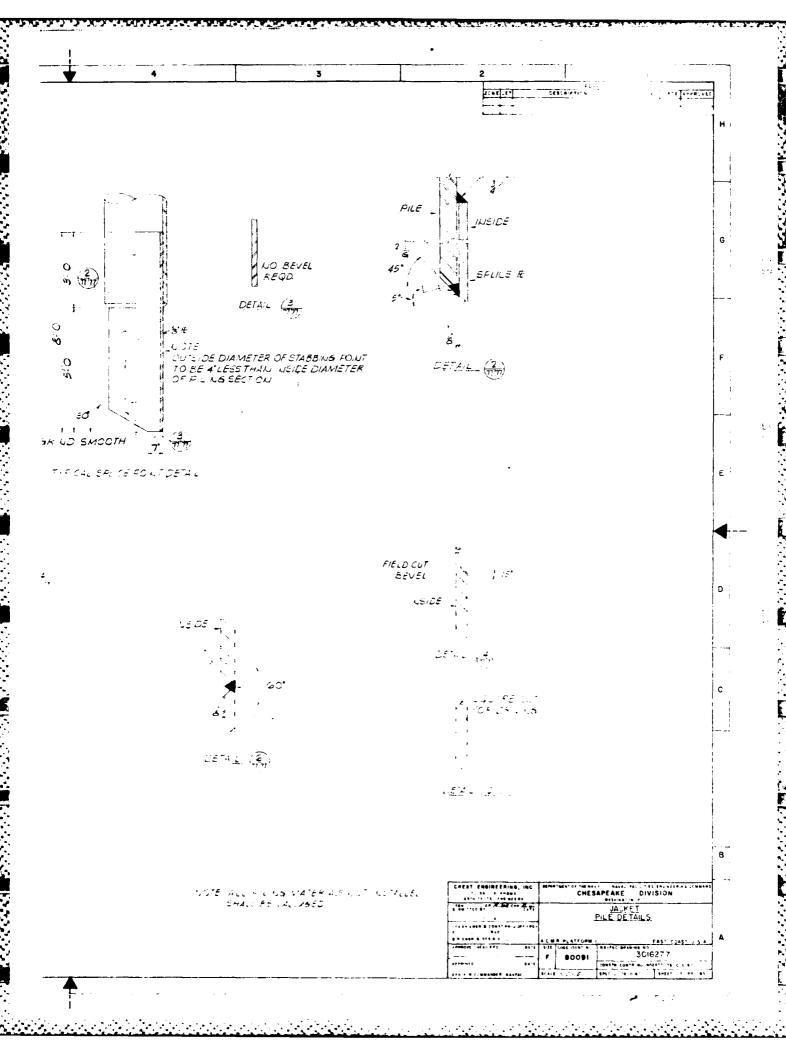


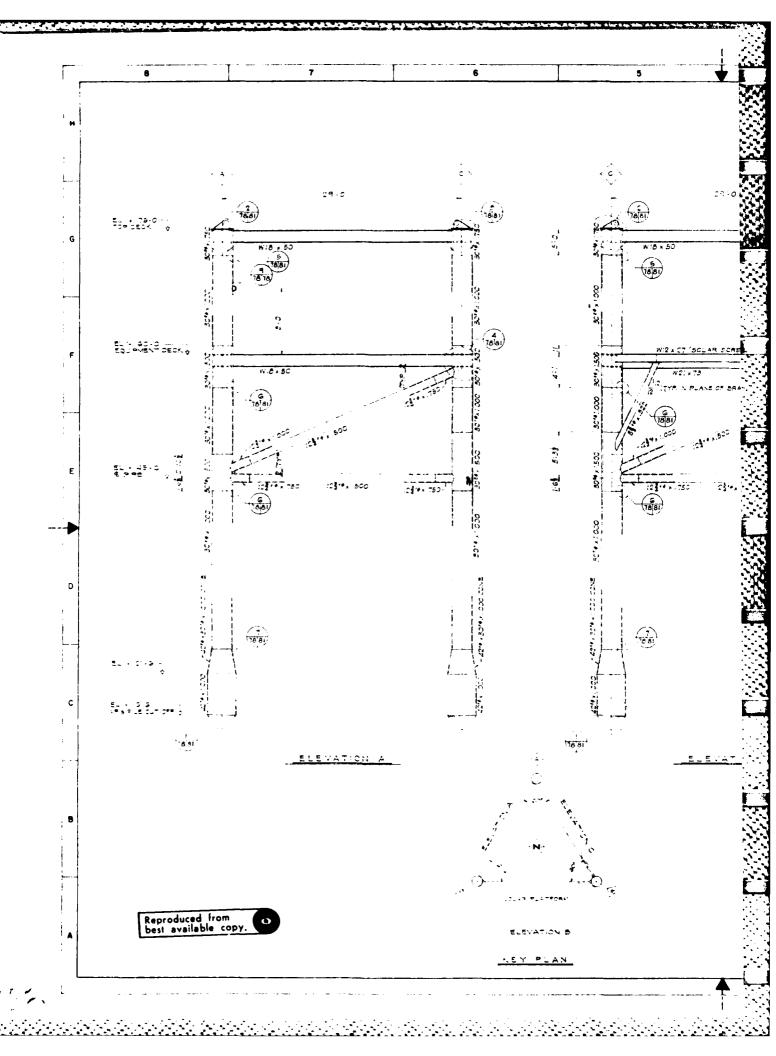


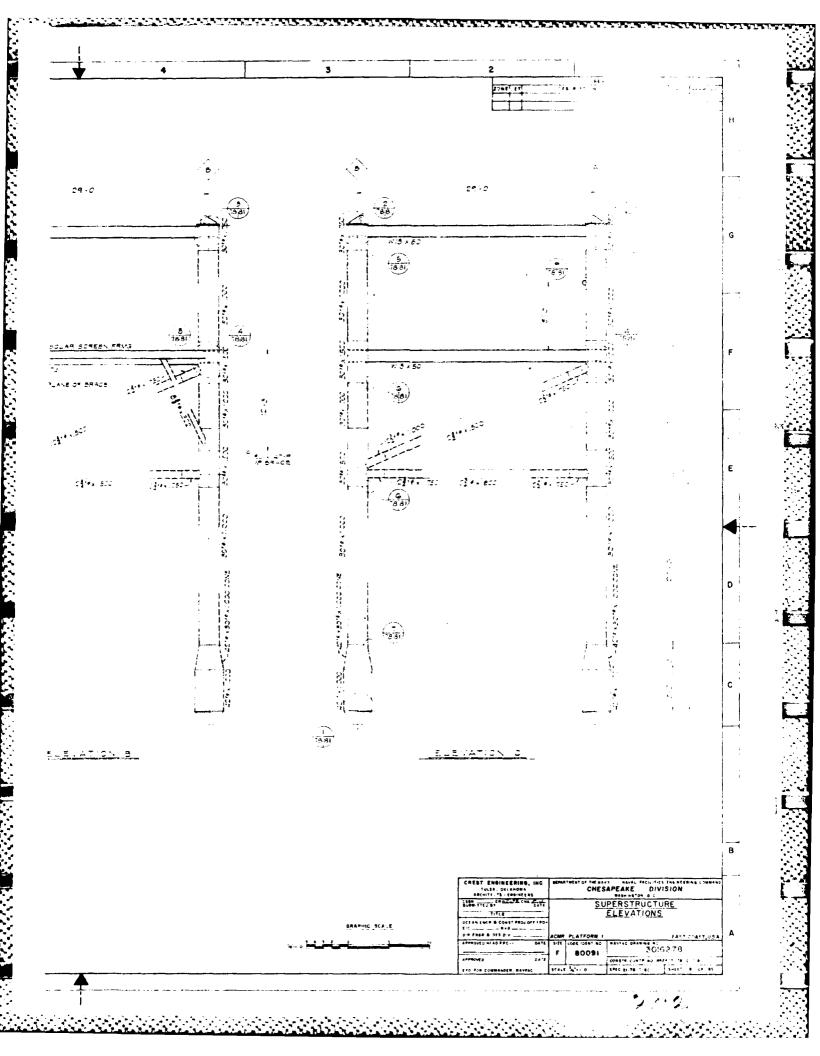


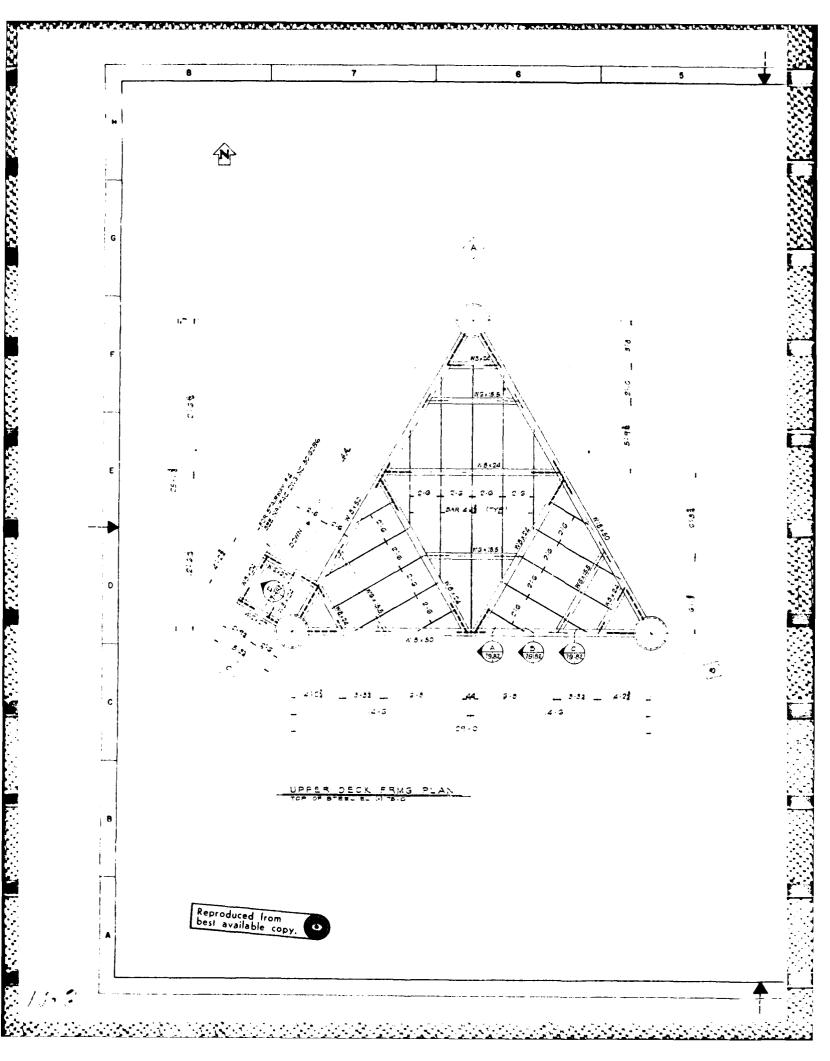


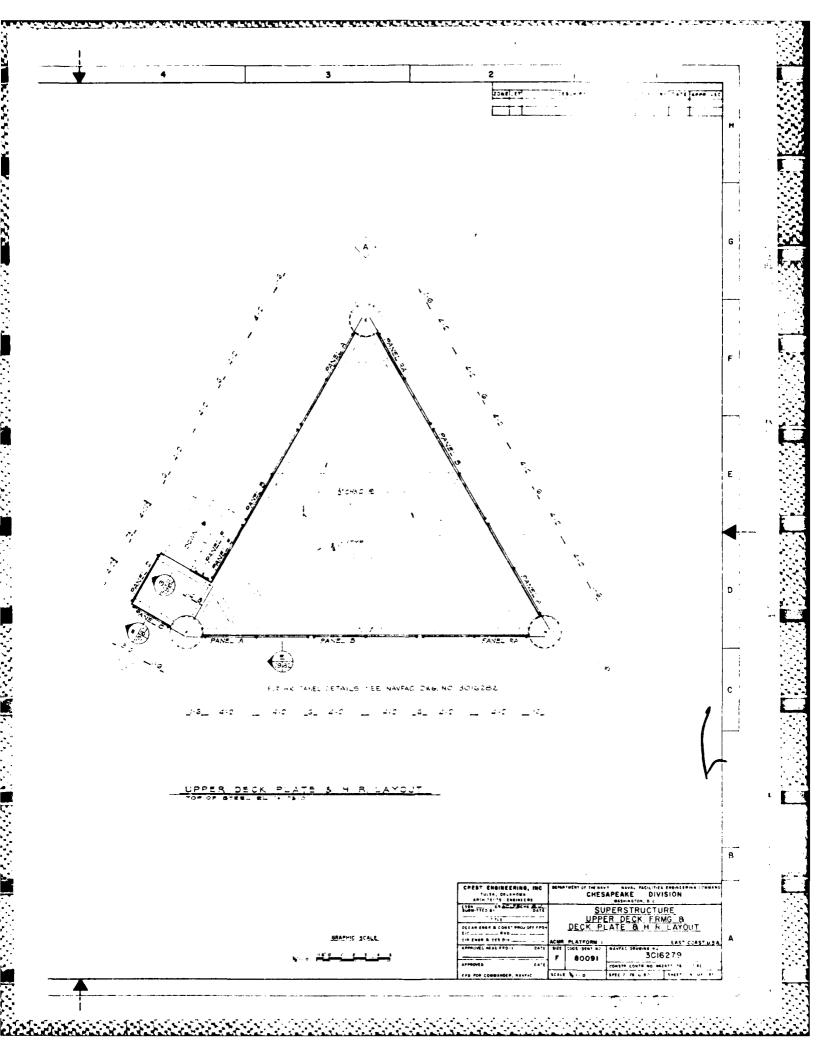


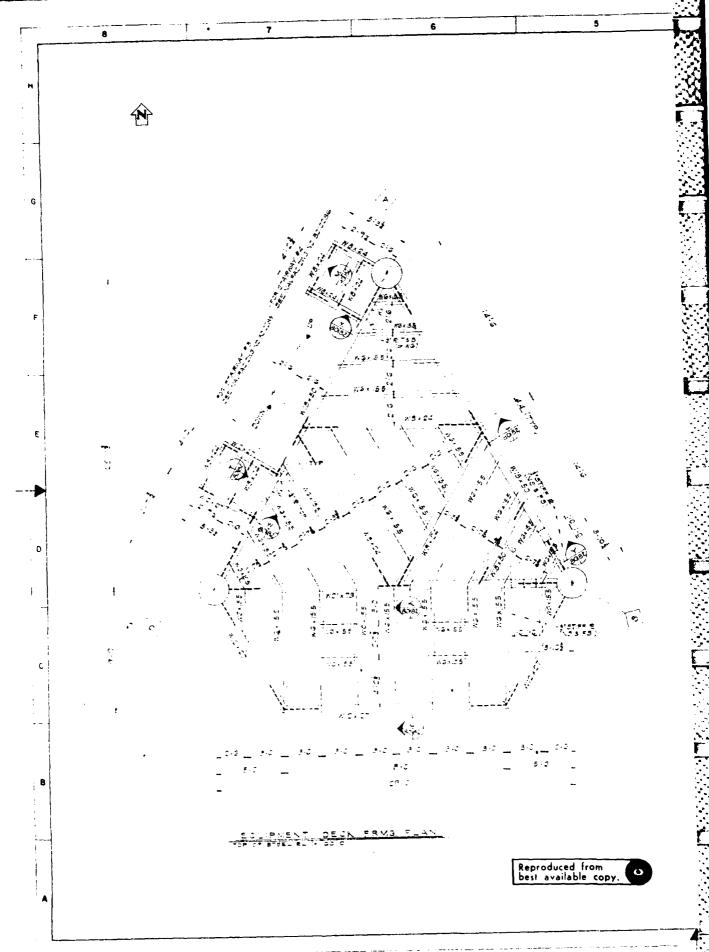


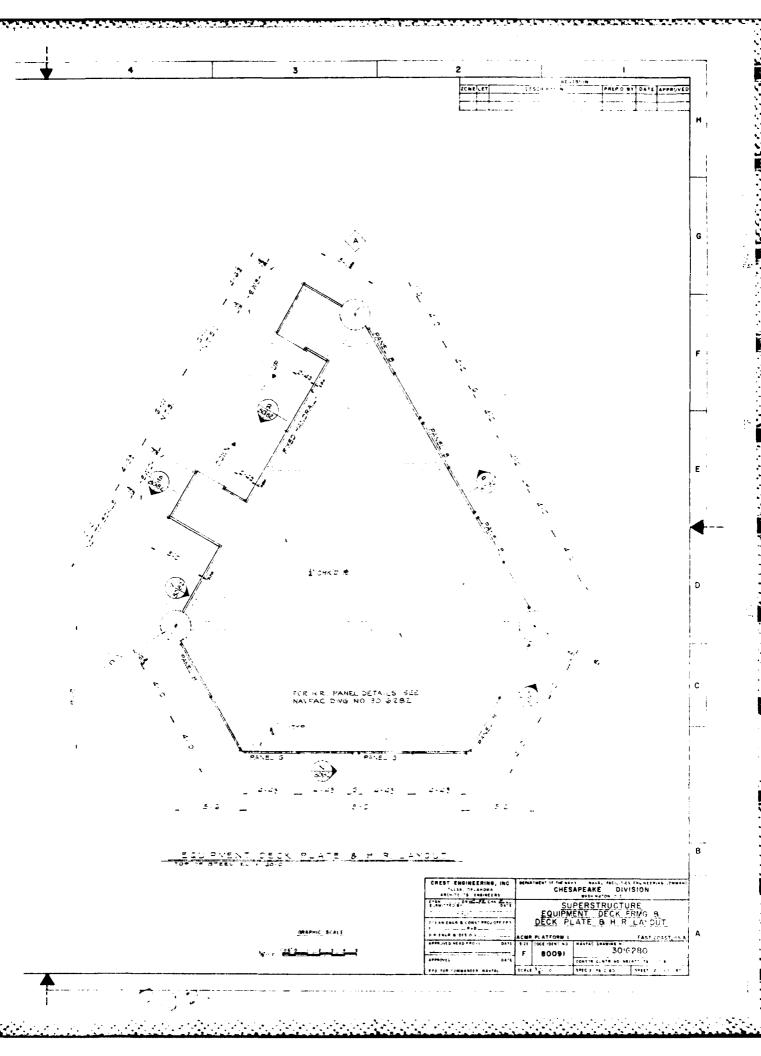












SECTION 3.0
STRUCTURAL IDEALIZATION

3.1 INTRODUCTION

This section presents the mathematical structural model used for the analysis of the 81 feet MLW structure.

The structure is modeled as a space frame. Joint coordinates and member incidences are generated, as illustrated in Section 3.2, to obtain an efficient computer model. The model is then used in the SEALOAD program to generate the wave loads applied to the structure during the 50 year storm. Finally, the model is used in the STRAN, program with the wave loadings produced by SEALOAD to analyze the structure for the 50 year storm.

To fully represent the jacket's structural behavior, dummy members are used to simulate the pile-jacket interaction. These members are modeled so that only shears perpendicular to the piling are transferred between the jacket and the piling.

The pile-soil interaction is considered in STRAN through the Coupled Interaction Analysis feature. This achieves convergence between the boundary conditions of the nonlinear pile foundation and the linear structure. The input data required for this feature is found in Section 3.5.

In STRAN the individual structural members of the mathematical model of the structure are not given distinct integers for identification. Each structural member is identified by the joint number at the beginning of the member and the joint number at the end of the

member. Therefore, Member 701-703 is that member of the model connecting Joint 701 to Joint 703. The member start is Joint 701 and the member end is Joint 703, and therefore, the local (member) x - axis is positive toward Joint 703.

Also in STRAN, member properties are designated through GROUPS.

Each GROUP has a unique set of member properties, and each member of the model is assigned to a particular GROUP with the member incidence card. A list of the GROUP designations is found in Section 3.3. The member properties of each GROUP are listed in Section 3.7.

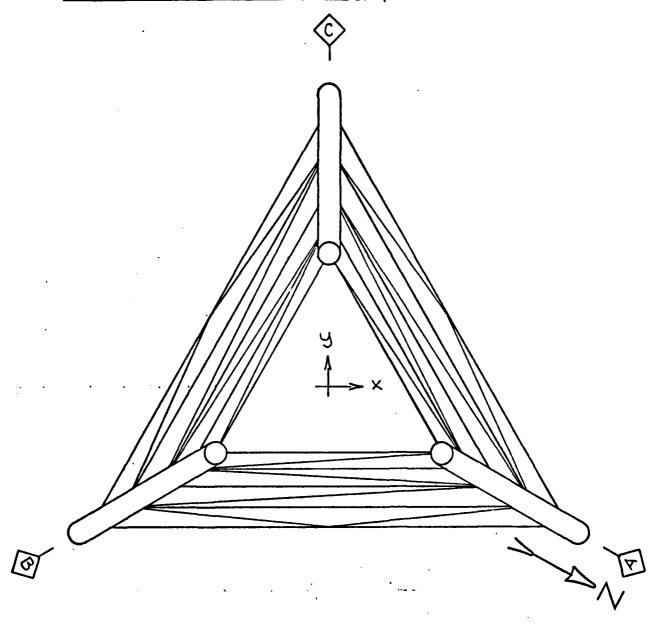
Reference Drawings:

3016265	Jacket - Elevations
3016266	Jacket - Plan at El. (+) 12'-0"
3016267	Jacket - Plan at El. (-) 13'-0" & (-) 47'-0"
3016268	Jacket - Plan at El. (-) 81'-0"
3016278	Superstructure - Elevation
3016279	Superstructure - Upper Deck Framing
3016280	Superstructure - Equipment Deck Framing

Sheet _ _ of _13 __

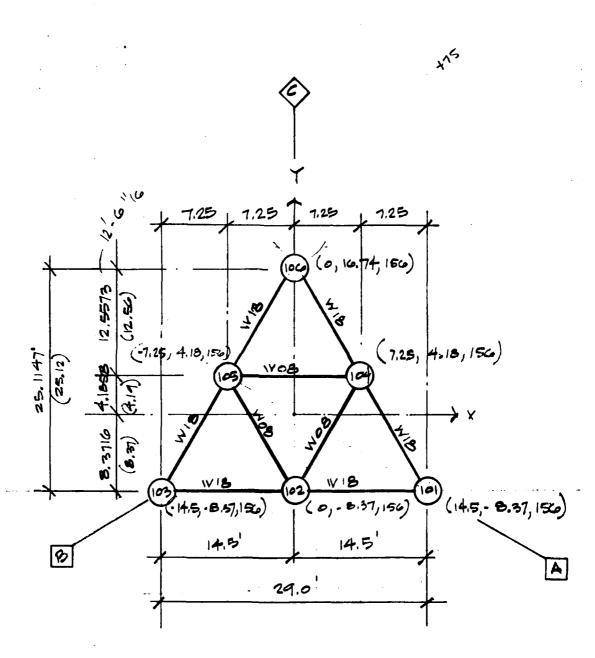
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3.2 SKETCHES - PLANS ? ELEVATIONS



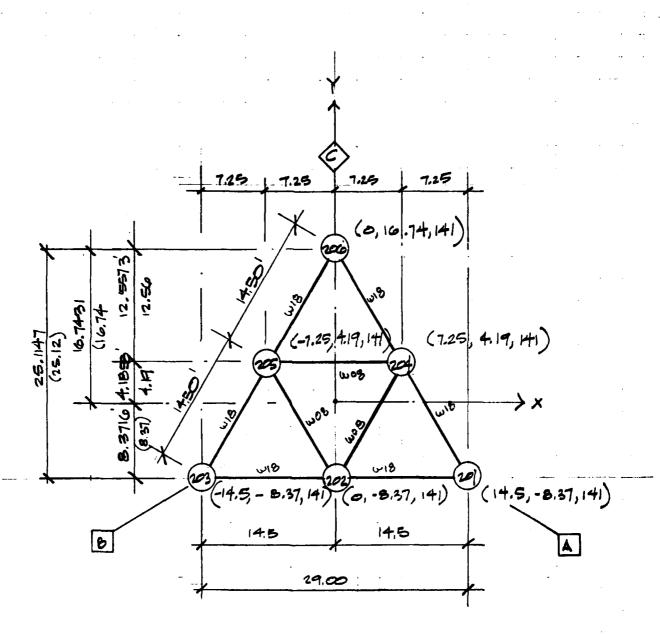
KEY PLAN

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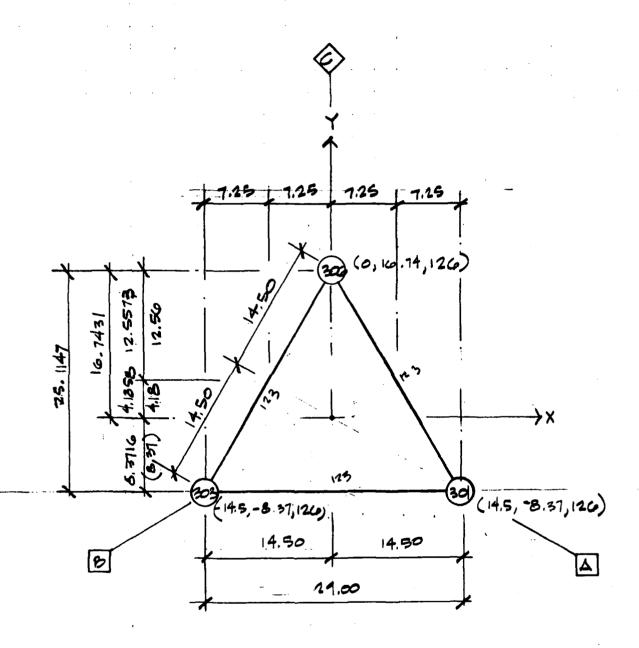
UPPER DECK (+) 75'-0 (156.0)

Sheet 205 of



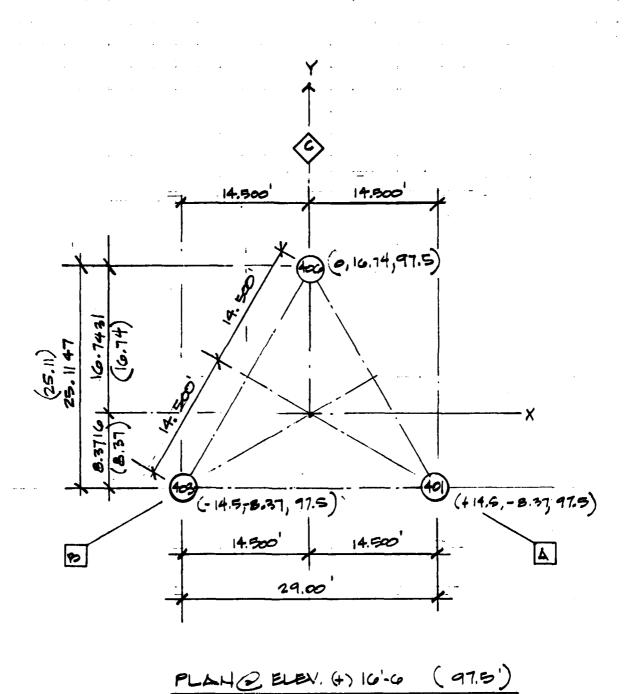
BRACIHG @ ELEV. (+) 60'-0 (141.0') BOUIPMENT OBCK

By Jan _ Client O. Myy _ subject _ my of DI inchine _ Date Co. 2:70 Job No. 21-171-94 _ Calculation Zanduril | de de 3:18 _ ___



BRACING CELEY. (+) 45'-0 (126.0')

By _____ Client O_S_ NAUY ____ Subject Teaign of SI MLW Systeme ______ Date Ce1274_ Job No. 27-771 = 94 ___ Calculation ______ Included less than 100 me.

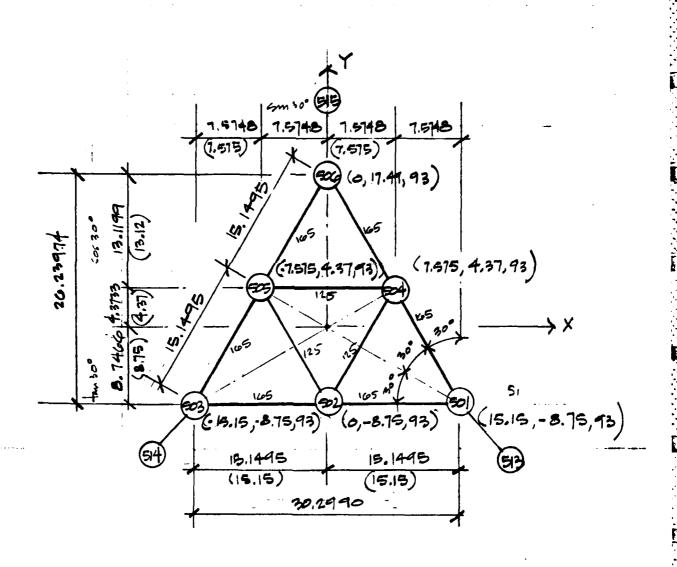


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Sheet 5.02 of ____

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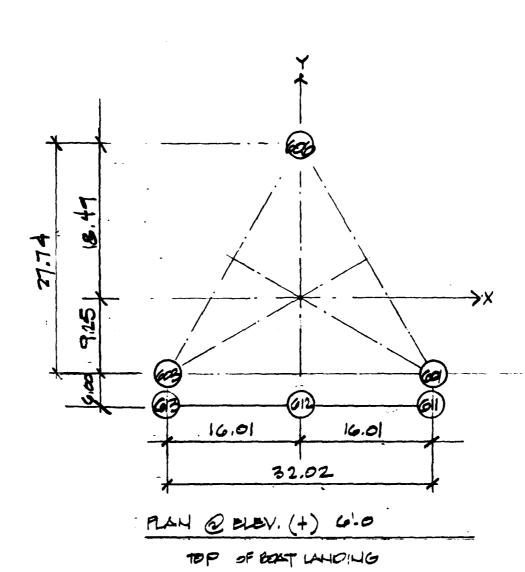


BRACING @ ELEV. (+) 121-0 (93.00)

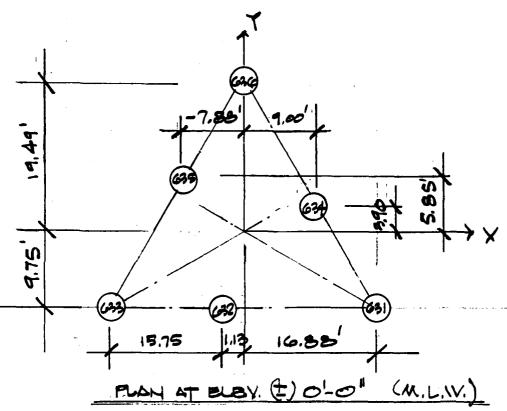
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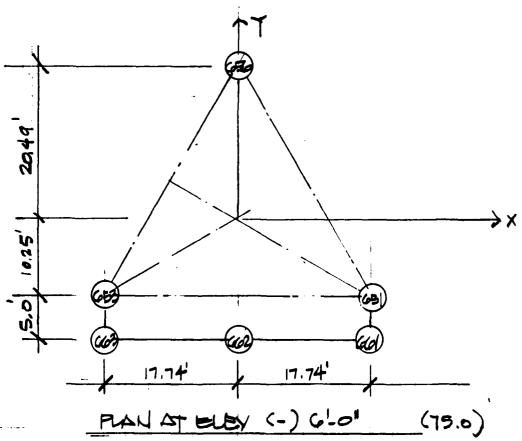
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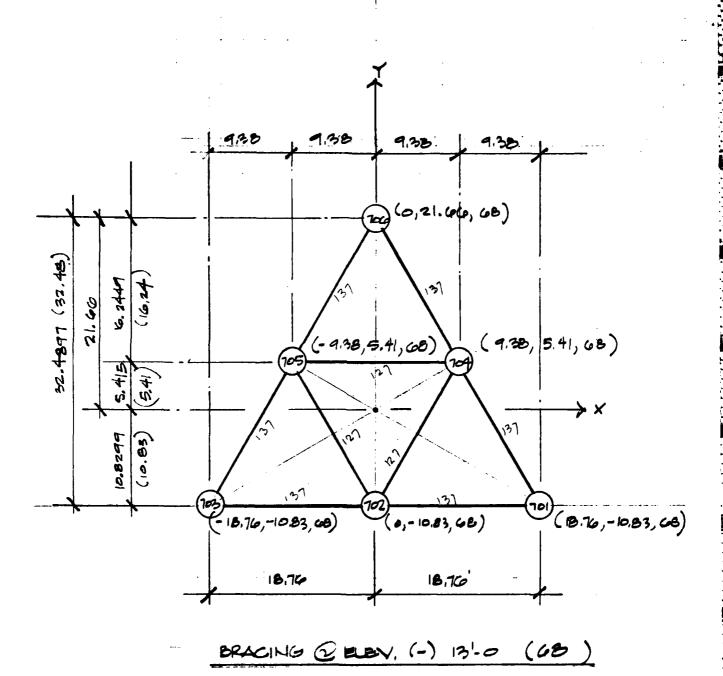
Sheet 31 of ____





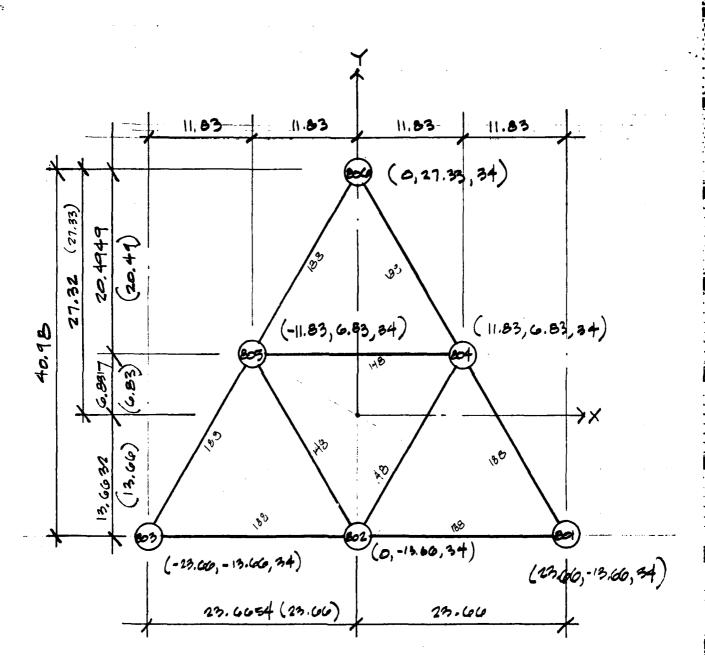
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Sheet 3.12 of .



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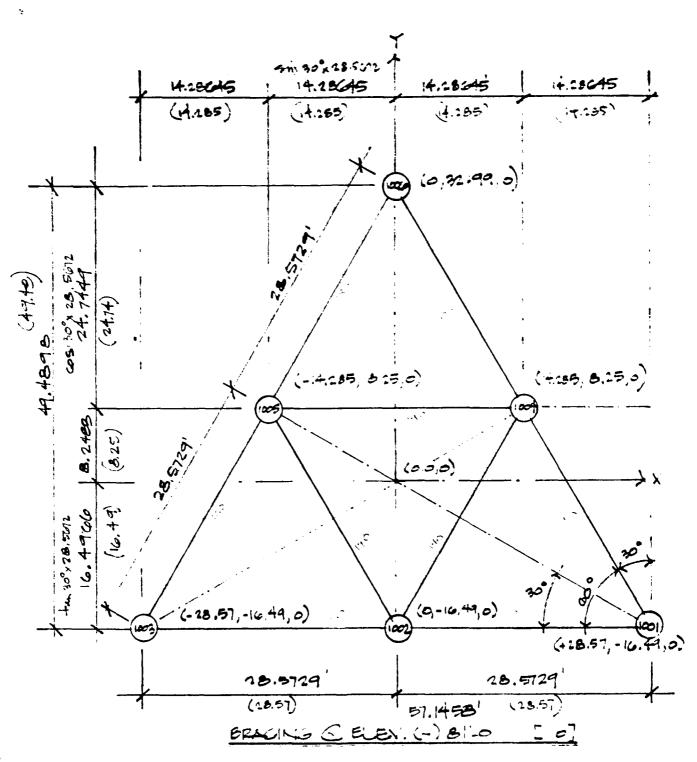
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SECTION 4.0
BASIC LOADS

4.1 INTRODUCTION

This section presents the loads which are applied to the structure.

Section 4.2 contains the estimated weight of the structural material not considered by SEALOAD because of the structural idealization of the model.

Section 4.3 contains the calculations for live loads applied to the Upper Deck and to the Equipment Deck.

Section 4.4 illustrates the data required for the wind loads feature of SEALOAD. The wind loading applied to the individual structural members of the model is found in Appendix B.2.

Section 4.5 contains a summary of the shear force and overturning moment at the mudline for each wave direction. The wave loading applied to the individual structural members of the model is found in Appendix B.2

CREST OFFSHORE, INC.

\$1.02 Sheet of

By VITalket_ Client_U.S. Navy __ Subject Design of 81- Min Structure__
Date_7=Z-76 Job No. 27-771-94 _ Calculation _ _ Basic Loads _ _ _ _

4.1 Dead Loads

The weight of the structure is computed by the SEALOAD-Z Program for the structure represented by the mathematical model. For that port of the platform that is not described by the model the following loads were added to the Dead Load, Condition \$3.26 Load Condition \$6.

Top Deck

15.0 * Total - Estimated

Distribution: 5.0 * to each Column (A,B,C)

Equipment Deck

18.0 Total - Estimated

Distribution: 5.0 to to Column A

6.5 to Column B&C

Boat Londing

22.0 total - Ref. Miscellaneous Structures

Distribution: 11.0 to Center of Member 601-611

11.0 to Center of Member 603-613

Boot Bumpers

2.4 Total - Estimated

Distribution: 1.2 to bumper at Joint 513

1.2 to bumper at Joint 514

Sheet ___ of

By V_ Talbat client_ U.S. Navy _ subject Design of 81 Mill Structure __
Date_7-Z-76 Job No. 27-77L-94 _ calculation _ Basic Loads _ _ _

4.2 Live Loads

Top Deck

Live Load = 100 psf

Total Load = 0.10 ksf x 364 ft = 36.4 k

Load / Deck Beam = $\frac{36.4}{3}$ k = $\frac{1}{29.0 + 1}$ = $\frac{0.42 \text{ k/ft}}{3}$

Applied to Members: 101-102
102-103
103-105
105-106
101-104

Equipment Deck

Live Load = 150 psf

Main Deck Load = 0.15 Ksf x 364 ft = 54.6 K

Solor Panel Confilerer Load = 0.15 tsf x 192 ft2 = 28.8 x

For Members: 201-202; 202-203; 201-204; 204-206

Load / Deck Beam = 54.6 x 1 = 0.63 K/4+

For Members: 203 - 205; 205 - 206

Load / Dect Beam = (54.6×1) + 28.8×1 = $1.60 \times / 1$

CREST OFFSHORE, INC. By _ MO _ Client O. S. MANY _ _ Subject Design of BL MW Structure
Date _ 9.6.76 _ Job No. _ 27=771-94 _ Calculation _ _ _ _ 4.3 WIND LOADS The wind loads on that part of the idealized platform not in the wave are computed by the SEALOAD-2 Program. Apporten ances structure not contained in the methemetical model ere added as separate, a reas. The following mez was add for the analysis of this platform Segment #1-Soler Panel & Equipment Area = 15'x (29'+ 7.67) x0.5 = 290 P +75 VEFT. CENTROID ShowE Mudline = 1425' Wind Velocity = 145 M.P.H. +45 Sequent #2-Antenna +12' The enterme for this structure is shielded by the experstructure leg and therefore is neglected. -13 TOTAL WIND LOAD = 31.98 K -47'

4.05

By V. Talbat_ Client U.S. Navy ___ subject Design of Bl MLW Structure
Date 7-7-76 Job No. 27-771-94 Calculation _ __ Basic Loads _ ___

4.4 Wave Loads

The wave loads on the members of the idealized platform are calculated by the SEALOAD-2 Program using Dean's Stream Function wave grid profile.

A summary of the shear force and overturning moment at the mudline for each wave direction selected is included in this section. Note that these forces and moments also include the wind loads.

the following four pages contain the wave summary of Dean's Stream Function wave grid profile with the modification for free surface effects.

CREST OFFSHORE, INC.

Sheet 4.000 -

By J. Talkat Client U.S. Navy _ subject Design of _ MLW Structure

Date 9-3-76 Job No. 27-771-_ _ Calculation _ Wave Loads _ _ _

The roughness effect of the morine fouling for that part of the structure from the Mean Low Water to the Mudline is considered by increasing the effective diameter used in SEALOAD to increase the drag. However, this results in a larger inertial force being applied to the structure. Therefore, the mass coefficient is reduced correspondingly. The following equations are used to determine Deff and Cm used in SEALOAD.

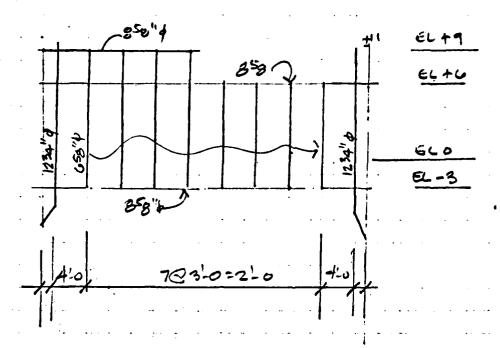
These equations produce the following table.

D	Dact	Deff	_Cm_
103/4"	12.75	17.57	0.257
1234"	14.75	20.33	0.297
14"	16.00	22.05	0.322
16"	18.00	24.81	0.363
18"	20.00	27.57	0.403
20 1	22.00	30.32	0.443
45 124	47.50	65.47	0.957
472	49.50	68.23	0.997

By _____ Client ____ Subject___

Date _ 51 2 12 Job No. _ 21 - 771 - _ _ _ Calculation

BOAT LANDING WAVE ACEA



HOPPENT FACE
HOPPENTALLS
1.889 = 0.72 \$\frac{1}{2} \times 16

1-884= 0.72 1 x 16 = 2-11 = 0.72 x 29 x 2 =

VEFT. 2-124 = 1.0025 x2 x 18' = 88.34 4-68' = 0.552 x4 x 13' = 28.70

4-" = 0,552 x4x 10' = _

11.52 F 41.70 / 53.20 38.34

28.70 22.03 / 89.12

BACK FACE

HOP. 1-68" = 0.552 × 16 ×1 = 8.83 5.F.

2-88"4 = 0.720 × 29×2 = 41.74"

VER. 3-689=0,552x9x2= 9,94 "

2-689=0.552x13x2 = 14,35

ASSUMING BACK FACE IS SHIELDED CONSWHIT

TOTAL AFEA = 142,4+ 0,5 x 74.88 = 179.84 SIF,

EAY 180 S.F. SUFFACE ALLY

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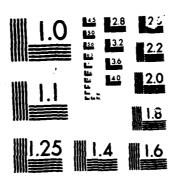
SECTION 5.0 LOADING CONDITIONS

5.1 INTRODUCTION

This section describes the wave approaches considered and the loading conditions used to analyze the structure for the 50 year storm.

Load Conditions 1, 2, 4 and 5 are the load conditions generated by SEALOAD for the maximum force on the structure (in the area of the wave crest) for the four selected wave approaches. Load Condition 3 is the dead weight generated by SEALOAD. Load Condition 6 is the dead weight not included by the model, and the live load on the two deck areas. Load Conditions 7 to 10 are the maximum wave load conditions, Load Conditions 1, 2, 4 and 5, added to the sum of Load Conditions 3 and 6, the total dead weight and live load of the structure.

DESIGN CALCULATIONS 81' MLW STRUCTURE EAST CORST AIR COMBAT MANEUVERING R. (U) CREST ENGINEERING INC TULSA OK SEP 76 27-771-94 N62477-76-C-0179 ND-A165 698 2/8 UNCLASSIFIED F/G 13/13 NL

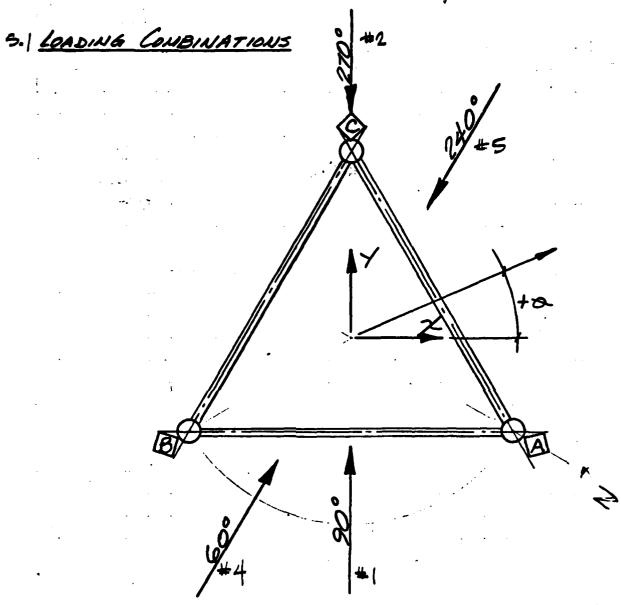


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MICROCOPY RESOLUTION TEST CHART

CREST OFFSHORE, INC.

Sheet 5.02 of By_ MM_ CHEM_ U.S. MAYY _ Subject Design of BI MLW Structure Date 7-9:76_ Job No. 21-171-94 _ Calculation _ Loading Compensations



WIND and wave of 90 LOAD GASE 1: WIND and whole of 270° LOAD CASE 2: LOXID GASE 8: DEAD LOADS BUAN FOR CIVIN LOAD CASE 4: WIND and WAVE ++ 240 LOAD GASE 5: LIVE LOADS + EQUID CL. DEAD LOADS LOAN GASE G: LD 1 + LD. 3 + LD. 6 LD 2 + LD 3 + LD 6 7: LOAD CASE 3: LOAD CASE LD 3 + LD 4 + LO 6 9: LOAL CASE 5 +406 10: LOAD GASE

SECTION 6.0

SPACE FRAME ANALYSIS

6.1 INTRODUCTION

This section contains the results of the space frame analysis of the structure subjected to the specified environmental conditions.

The space frame analysis set forth herein utilizes the available computer programs available at Synercom Technology, Inc., Houston,

Texas. The program processing procedures are as follows:

- Set up SEALOAD-2 program to obtain desired wind, wave and dead weight (including buoyancy effect) loadings on the structural components.
- 2. Update loadings in Step (1) due to additional dead weight and live loads on the structure.
- 3. Perform space frame analysis by using STRAN computer program.

Sheet Con of ____

62 MXMUM MEMBER STRESSES

THE FOLLOWING THREE PAGES TABULATE
THE MAXIMUM STRESS EACH MEMBER OF
THE MATHEMATIKAL MODEL EXPERIENCES.
THE LOADING CONDITION IN WHICH THIS
STRESS OCCUPS AS WELL AS THE MAXIMUM
UNITY CHECK ARE INDICATED. SINCE
A ONE-THIRD INCREASE OF ALL ALLOWARD
STRESSES WAS USED BY THE COMPUTER
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CREST OFFSHORE, INC.

By _ SWA_ CHEM D.S. NAVI _ Subject Desich & B! MAY SPUCIAE

Date 9.5:70 Joh No. 27:771-14 Colombian

6.3 MAXMUM MEMBER POPCES

THE ROLLOWING THREE PAGES TABULATE

THE MAXIMUM MEMBER FORCES FOR

EACH MEMBER OF THE MATHEMATICAL

MODEL. SINCE A ONE. THIRD

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10	12000	-6-50017	0.7770	07000	90000	A.2.F.O.O.		
· ~	67170	1957	-10215	90 u 0 0 °	2,0001-	99070		
63	*05+0°	-6,41452	-11443	05000.	01000*-	• 00306		
4 .	.10747	-5.5303	-12014	85000°	\$1000 ·	00110		
5	1457	9.47004	60511309	65000	\$1000	51000		
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203	21000	45.30805	5711.0	70000	21000	00317		
204	21770		· 15358	7,000	2/000	00100		
205	. 10048	-A. 40000	~	11000	**0000	07000		
0.5	10411	-5.42457	.02152	67000	.00012	-,00065		
301	£550.		0/2/3	.00500	10000	00422		
0.3	# 22C0 *	-6,14209	-,10483	00346	-,0007	-,00356		
300	.00058		. UZ10H	60000	\$\$000°	-,00032		
401	06165	-3.42808	05842	25000	61000°-	29000.		
808	\$1470.	-3,43702	00100	, 00412	0000	.00150		
€ ⊃ ₹	> T 4 7 3 * I	-3,77393	.01500	.00354	- 0000 P	£0000°		
501	P7730.	-3,70397	** 0027b	.00377	60000	£1000°		
205	.05067	-3.75686	-,15567	.00257	* 100012	• 00050		
	750/0	-5.74105	05401.	15500	E 5000°	81100		
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106	04500-	-3.40HOO	115UH	.00071	77000	95000		
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3	** 60.104	-5.44619	- 20617	100157	B 0000	26170		+
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102	2.00454	5,50534	-12654	65000	000047	50000		
103	2.60540	4.42549	97100	00027	07000	16400.		
104	1.07950	5.154UB	16453	£0000.	£0000*-	.00716		
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	8-402-8	さのトイコ。さ	- 00505	15000-	.00142	\$1700.		
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205	2.24535	3.14456	62200	00100	000050	.00651		
513	4.25219	2.0/170	.04159	00257	\$ 100.	50500		
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CREST OFFSHORE, INC.	Sheet 6.19 of
By_ WA_ Client U.S. 1447	Subject Deletion
Date 9:5:762 Job No. 27-771-54	Calculation

6.5 REACTIONS

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1012	3,6501	-350.4103	1412,3924	10507.0760	560.0032 50.0032 096.5194	-361,4634 -3412,5600 -3650,1552	GLUMAL UMLTGUE GLUMAL	
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SECTION 7.0
TUBULAR JOINT ANALYSIS

7.1 INTRODUCTION

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This section contains the analysis of all of the tubular joints of the structure. The tubular joints are separated into two groups, the Primary Joints and the Secondary Joints.

The Primary Joints are those joints involving the jacket legs.

Section 7.2 displays the joint geometry and location for each of the Primary Joints. This section serves as a key to Section 7.3, the computer analysis of the Primary Joints.

The Secondary Joints are those joints involving the interior bracing at each of the horizontal levels. Section 7.4 displays the joint geometry and location for each of the Secondary Joints. This section serves as a key to Section 7.5, the computer analysis of the Secondary Joints.

The computer program used for the tubular joint analysis of this structure is a post-processor program for STRAN developed by Crest Offshore, Inc. This program is based on AISC and API criteria for stress in tubular members.

Reference Drawings:

3016265 Jacket - Elevations

3016266 Jacket - Plan at El. (+) 12'-0"

3016267 Jacket - Plan at El. (-) 13'-0" & (-) 47'-0"

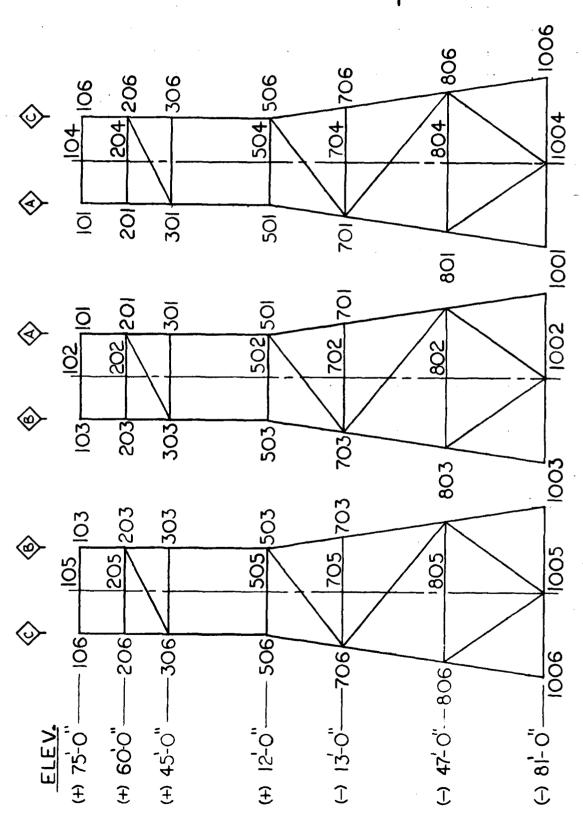
3016268 Jacket - Plan at El. (-) 81'-0"

3016278 Superstructure - Elevations

JOINT GEOMETRY - PRIMARY JOINTS

By L. Kirk Client U.S. HAVY _ Subject DESIGN OF 81'MLW STRUCTURE Date 8-18-76 Joh No. 27-771-94 _ Calculation TUBULAR JOINT ANALYSIS

7.2 Joint Geometry - Primary Joints

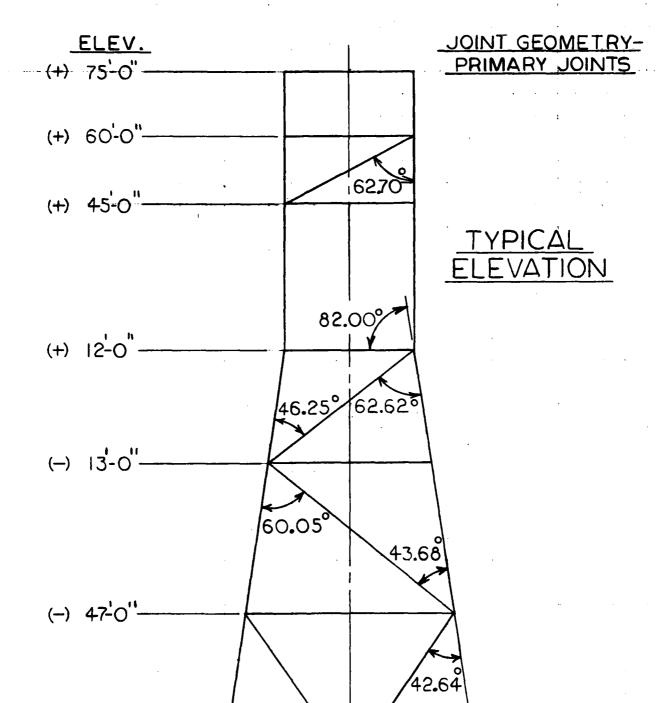


CREST OFFSHORE, INC.

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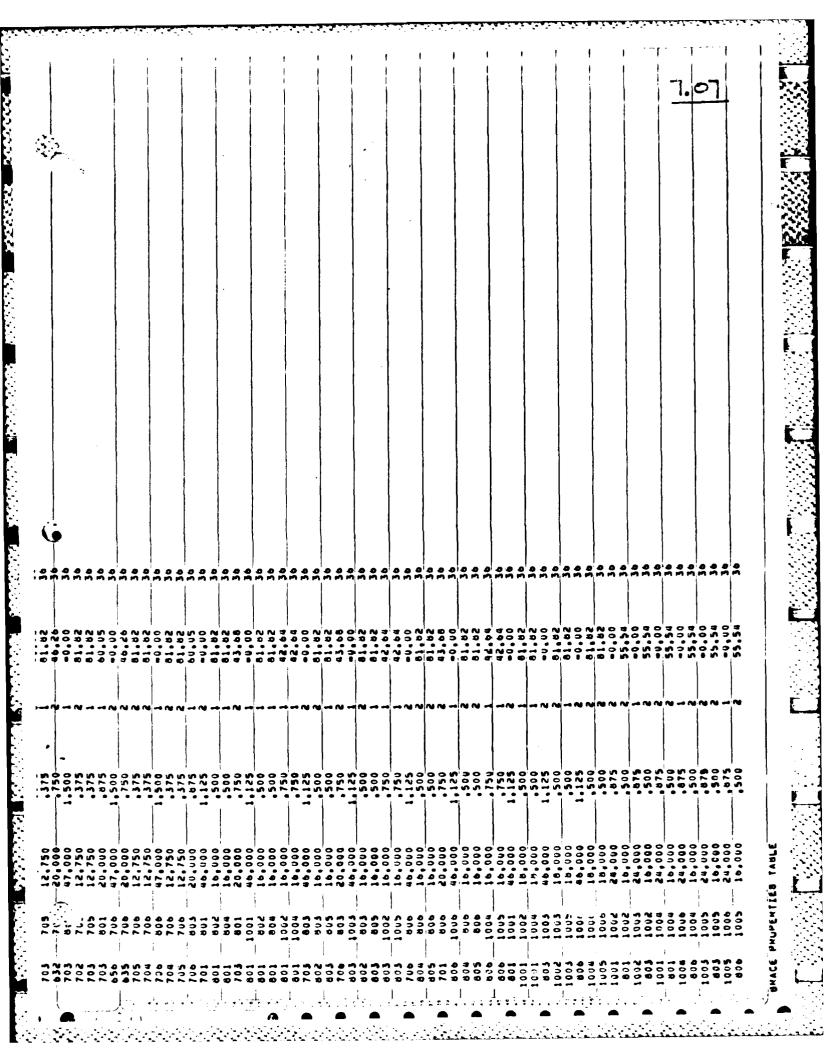
By L.KIRK Client U.S. NAVY Subject DESIGN OF 81 MLW STRUCTURE
Date 8-18-76 Joh No. 27-771-94 Calculation Tubular Joint Analysis

Sheet _____of __.



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Sheet 12 of .

By L. KIRK_ CHANY __ Subject DESIGN OF BI MLW STRUCTURE.

Date] -28-76 Job No. 27-771-94 _ _ Calculation Turunar Joint Analysis

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7.4 Joint Geometry-Secondary Joints

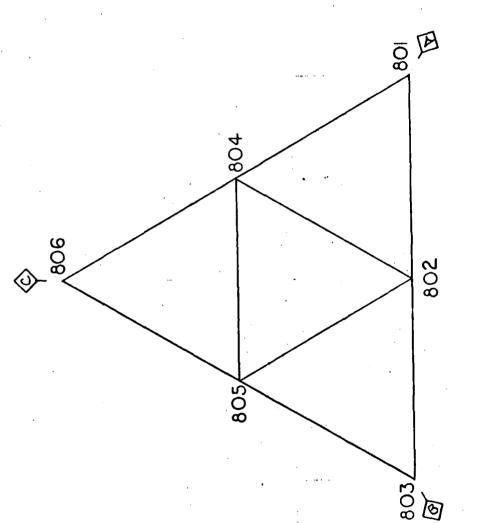
PLAN AT ELEV. (-) (3'-0"

705/

JOINT GEOMETRY - SECONDARY JOINTS

Sheet 7.25 of ____

By L. KIBK __ CHANT U.S. MAYY __ Subject DESIGN OF 81' MLW STRUCTURE
Date 7-28-76 __ Job No. 27-771-94 __ Colculation TUBULAR JOINT ANALYSIS __

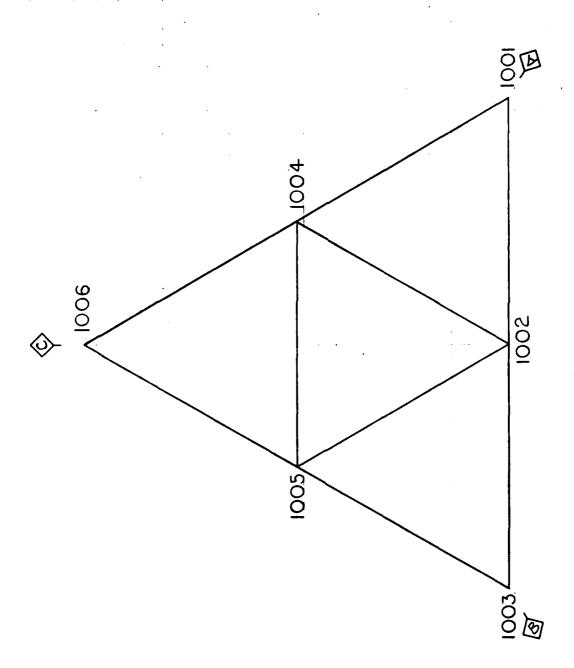


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JOINT GEOMETRY - SECONDARY JOINTS

By L. KIRK CHAN U.S. MANY _ Subject DEBIGN OF BI'MLW STRUCTURE.

Date 8-18-76 Joh No. 27-771-94 _ Calculation TUBULAR JOINT ANALYSIS _



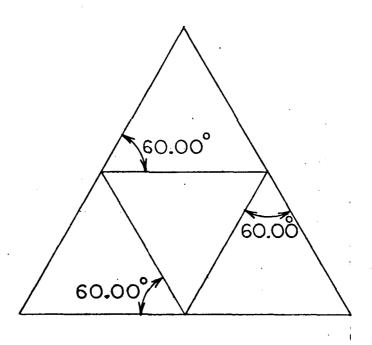
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JOINT GEOMETRY - SECONDARY JOINTS

Sheet ____ of ____

By L. KIRK _ Client U.S. MAYY _ Subject DESIGN OF 81' MLW STRUCTURE Date 7-29-76 Job No. 27-771-94 _ Calculation TUBULAR DOINT ANALYSIS _

JOINT GEOMETRY - SECONDARY JOINTS



TYPICAL PLAN

Sheet 7.28 of _

7.5 PUNCHING SHEAR ANALYSIS-BELON DARY JOINTS

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ANIAL	1.813	.035	.023	3.957	3.971	3.528	3.602	2.043	1,627	3.445	3.142	4.043	1.040	3,370
THICKE	.365	. 405 405	. 475 365	. 305	305.	. 305	.365	.365	. 365	.365	365	. 365	305.	. 365
DIAMETER	24.00	24.00	24.00	24.00	24.00	24.00	24.00	10.75	24.00	24.00	24,00	24.00	24.00	24.00
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SECTION 8.0
PILE-JACKET CONNECTION

8.1 INTRODUCTION

This section ascertains the capability of the pile-jacket connection at the top of the jacket of transferring both the axial load and the bending moment of the jacket to the pile. The following assumptions are made in this analysis:

- 1. The axial load is distributed to all six shims.
- The resultant bending moment is taken out as a couple by two shims on each side of the 42 inch diameter pile.
- The torsional moment is negligible.
- 4. The fillet weld area perpendicular to the applied load is more susceptible to fatigue crack than the fillet weld parallel to the load, and, therefore, is ignored in this analysis.
- 5. An E-70 electrode is used for welding with τ = 15.8 ksi or f = 11.2 ω kips/inch.

Reference Drawings:

3016270 Jacket - Pile Shims and Leg Connection

376,10 240.90

440.14

312.26

116.70 33.88 150.52

> 9808.15 11246.29

> > 1204.22

972/05 8621-16

-1240.94

0

1

4

-1653.90

11124.01

-1873.50 1644.86 103.2

523.44

12043.54

2447.12

-12404.40

940.63

127.60

10718.72

A 45.26 9048,33

125.4

10510.85

547603

130.5

19 362.79

18313.14

-628B.99 5508.co

430.23 488.12

1604.20 50%

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48.86

231.7

19463.72

-1840.18

6228.67

STORM

50yR.

6.2 JACKET LOADS of CONNECTION

M (In. - Kies)

 M_{γ} M_{Z} (IM.-KIPS)

(KIPS)

CONDITION 2007

MEMBER

4.8

JACKET

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TOTAL LOAD 40 381.18 378.33 385.55 299.49 371.10 **386.**4 (といと) 346. 156.07 250.13 10.46 18.33 64.60 55.0 (Kips) 158.44 **P/6** 158.64 MR/42"-2 (KIPS) 276.02

281.16

23017.5

2600,06 2313.93

-109.97

450.0 181.8

00

0

403-503

22185.71

-23039.5

-2599.35

227.78

19 133.19

18277.52

567.83

212.67

-5439,32 -16848.43 17863.47

274.14 20 0401 TOTAL LOAD IS MAXIMUM **BCOCA.**9 -918.09 NOTE .

SHIM PLATE **のこの**

Sheet 8.0301_

By MA __ Client U.S. MAYT __ Subject DEGICAL OF WIN KIN STRUCTURE __ Date & 5.70 Job No. 27-771-94 __ Calculation PLE-MCKET CONNECTION

8.3 CHECK JACKET TO SHIM

DINT CANTEN'S = 1.75"

EFFECTIVE WELD = 1.75 - 0.125 = 1.625" MAXIMUM

ALLEWARLE LOSO = 11,2 by x 1,625 = 18.2 by in.

REQUID SLOT LENGTH = 474 k = 26.04 m'
18.2 km = 24.04 m'
400 PRPHIMETER = 2×13/5 = 27 m > 26.04"
:. 0.k

8.4 CHECK SHIM TO PILE

JACKET - DINT CAN $\phi = 48''$ " TKHS = -3.5 = 1.75 K244.5

LESS PILE CHAR = $\frac{42.0''}{2.5''}$

ALL. SHIM IL TKNS .: 2 72 - 0,25 = 10"

MAX EFFECTIVE WELD = 10.88"

ALLONABLE LOND = 11.2 / 11 x0.88 "= 9.8 / 11

LEHOTH REQ'D = 474 = 24.2" < 31.0 m :. 0.k.

Sheet 3.04 of ____

By _MA __ Client US_LAY1 __ Subject FESSEL SE SI MULL SPOSTUSE
Date _8.5.76 Job No. 27-71-94 _ Calculation PILE-LASKET SONNETTON _

8.5 CHEK SHIM STRENGTH

SHIM WIDTH = 4.5" x 71 - 3.5 = 19.8" SAY 18"

SHIM AFEA = 18" x 1.00" = 18 Eq. ii.

SHIM STREES = 474" = 20.3 Hol 2 28.7 Kui.

18 m² = 016.

BIG CHECK JACKET LEG

= 1.75" × 20" = 35 m2

12 FET STREES = 474k = 13.6+kg < 28.7 kg = 0.k.

SECTION 9.0

PILE ANALYSIS

9.1 INTRODUCTION

This section determines the pile penetration and pile schedules of Structure 1.

First, the actual maximum pile loads are calculated. Then, these loads are used with the Pile Capacity Curves to establish the penetration required. Finally, the Pile Driving Resistance Curves are checked to insure that the piling can be driven to the desired penetration.

The pile schedules are devised to avoid any possible set-up problems while driving and to minimize field welding of the pile add-ons.

In addition, the P-Y curves are included in this section. This is the P-Y curve used in Section 6.0 for the space frame analysis.

The Pile Capacity Curves and the Pile Driving Resistance Curves are from the Foundation Analysis, Report No. 27-771-97.

Reference Drawings:

3016277 Jacket - Pile Details,

Caractera e suscentrar a principal de la fishica de la fishica de la fishica de la fishica de la fishica de la f	
CREST OFFSHORE, INC.	Sheet 7,02 of
By Client U.S.NAVY Subject DEDICH GEDICH	el'Mus
9.2 PILE SUMMARY	
FIPUCTUPE # 1	
FILE AXIAL LONG	
Maximum Compressive Load	2409k
Maximum Tension Load	1740K
PILING DIMENSIONS	
Outside Diameter	42 in.
Penetration Below Mudline (5, F. = 150)	211 f f.
Penetystian Betra Mulline (S.F. = 1.35)	

CONCLUSION

Maximum Wall Thickness

Minimum Wall Thickness.

The pile schedule with 1.50 min. wall thickness can be drown to the required penetration of 211 ft. to obtain a safety factor of 1.5. No insert pile should be required

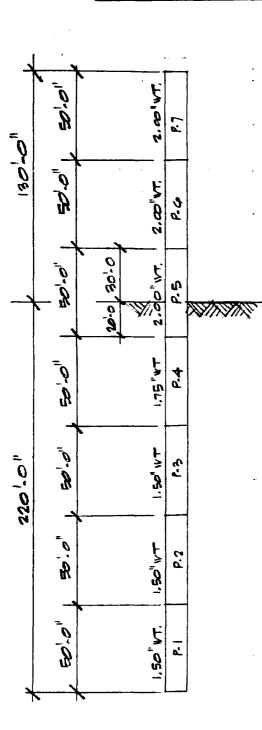
2.00 m

1.50 m.

9.03 Sheet ___ of _

By _ WA __ Client U.S. NAVI __ Subject FESIGN OF SI MIN STRUCTURE Date & 5:70_ Job No. 21:711-94 _ Calculation _ PILE EURYSS

93 ALELONDS - STRUCTURE # 1



MLY = BI'O"

220 FT PENETRATION

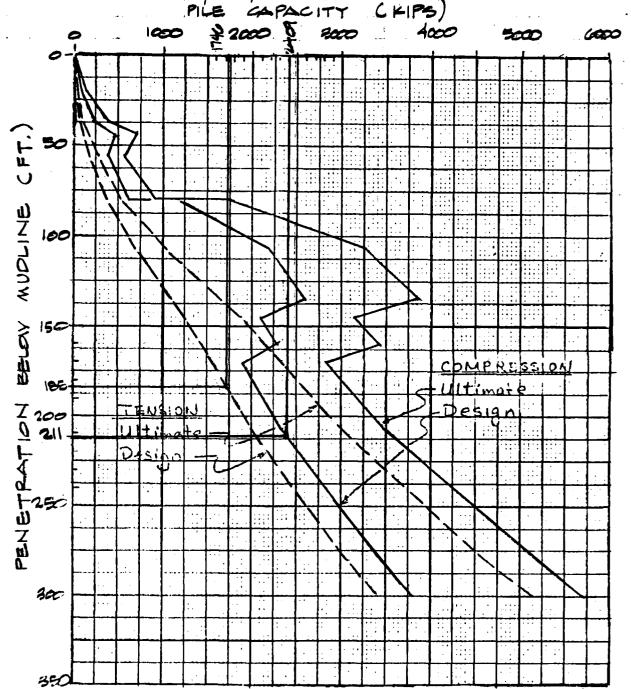
1.5" WT. MIH.

100 of 12" dx 2.0 = 0.854 \$ 1 x20 = 17.08 \$ 50 of 11" x 1.75 = 0.7523 x = 37.002 150 of 11 " x 1.50 = 0.049 x 150 = 97.35 152.05 \$ +splice points = 0.270 x 8 x 6 = 13.25 105.20 \$

MAX. COMPRESSIVE POPCE 2244 + 165 = 2409 +

MAX, TENSION FOACE 1871-165x+45 = 1746 By _WA _ _ Client_US NAVY _ _ Subject_ FESCH OF BI MLW STROCTURE Date B-6-76 Job No. _27-77L-94 _ Calculation _ PLE CURVES _ _ _ _

9.4 Ple Capacity CoruE



42-INCH DIAMETER PIPE PILES

BOKING # 1-8| T. MLIV LOGATION

Sheet 9,05,0f ____

By C. Chexil client U.S. NAUY __ subject _ DESCON of &1 Mcal Structure

Date 6-25-26 Job No. 27-771-94 _ calculation Pile Driving D'ailance Cours

9.5 DRIVING RESISTANCE CORVES

MLW = 81'-0"

250 FT Penetration

PLE SCHEDULE USED OF DRIVING RECEIPANCE

Vulcan 560 Hammer
Wt. of Ram = 60,000 lbs
Rated Energy = 300,000 ft-lbs
Hammer Efficiency = 0.75
Wt. of Pile Cap = 42,000 lbs

Spring Constant = 6.2 x 10⁶ lbs/in.

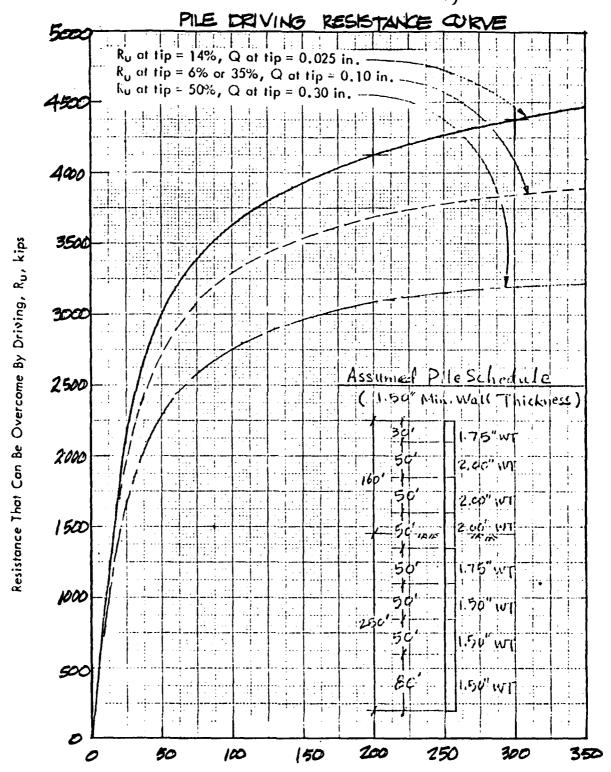
Damping Factor, side & tip, J = 0.15

Quake Factor, side, Q = 0.10

Quake Factor, tip, - See Above

160'-0"	50-0" 50-0" 30-0	2.00"WT 1.76"4 P.7 P-8
160	50-0	2.00"W
_	20.0"	ΔΤ <u>Ψ.2.eo"</u> <u>1.eo"</u> <u>Ψ.</u> P – S P – 6
	50:0"	1.75"WT P-4
,,	,-0" 50-0"	6"NT 1.50"WT 1.75"WT
9	ő	5
250,-0"	50'-	1.50.

By G. Chem Client U.S. NAVY __ Subject Design of 81 MCW Structe
Date G-28-76 Job No. 27-771-94 _ Calculation Lie January & Market & Ma



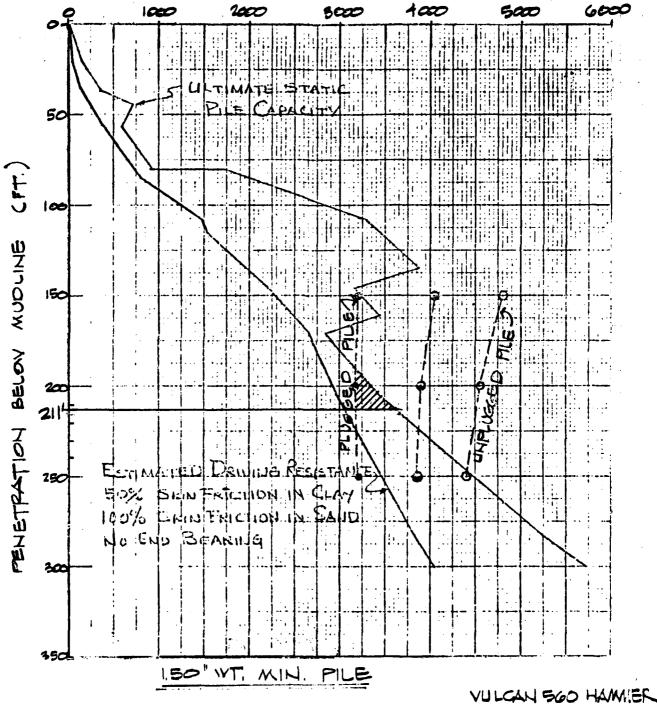
Rate of Penetration, N, Blows per Foot 250-Ft Penetration

By _WA __ Client_USNAYY _ _ Subject_FESIGN OF SIMLY STRUTUPE

Date & GIQ Job No. _ 27-771-94 _ Calculation _ FISE CAPACITY _ _ _ _ _

BUT MATE STATIC PILE CAPACITY (KIPS)

BUT MATED DRIVING RESISTANCE (KIPS)



42 - INCH DIAMETER PIPE PILES

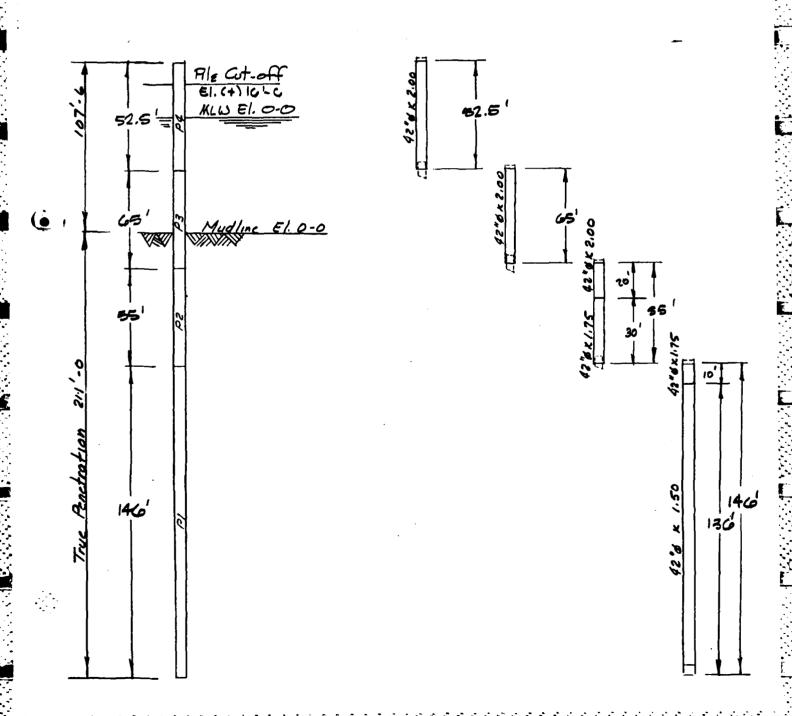
BORING # 1 - BI T MUY LOCATION

Sheet 9.08 of ___

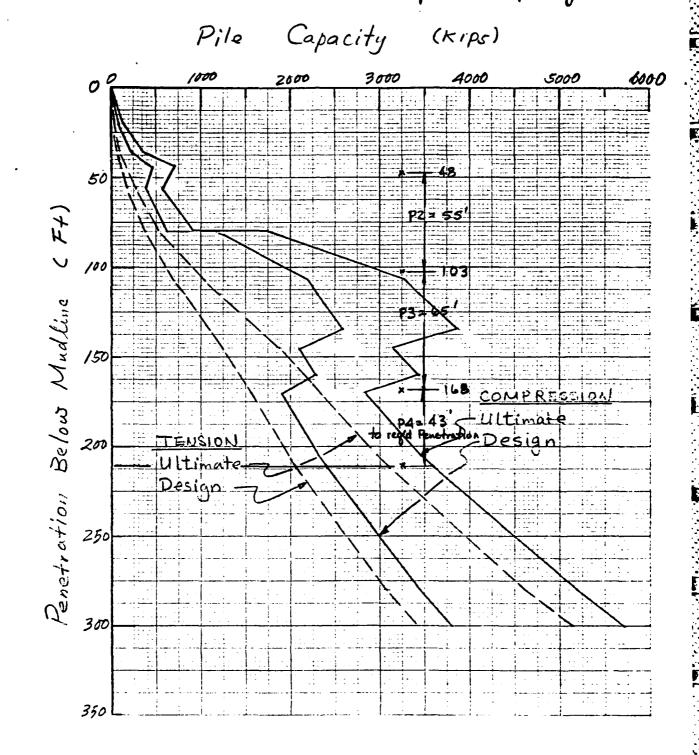
By V. Talbot_ client_U.S. Navy __ _ subject Design of 81' Mehl Structure __
Date 8-23-76 _ Job No. _ 27-771-94 _ calculation _ _ Ble Analysis _ _ _ _ _ _

9,6 PILE SCHEDULE

Site #1



By C. Shern client U. S. NAVY _ subject Structural Concept Analysic 3-pile Date 4=1=76_ Job No. 27-771-92_ calculation Pripe Pile Capacity Curves_



42-in. Diameter Pipe Piles (Boring #1)

Sheet 9.09 of _

CHECK MAX, LENGTH OF PLE ADD-ON

weight of Hammer c/ Leads = 230k

Using an import factor of 2.0,

Total Vertical Load = 400k

Weight of Piling (42" + x2.00") : 0,071 = 1

Assume L: 67': 804 in.

To hammer = PL = 17 x 804 m = 25.8 kg

for paling = w12, oph 4/m (804m.)2 = 1.9/m

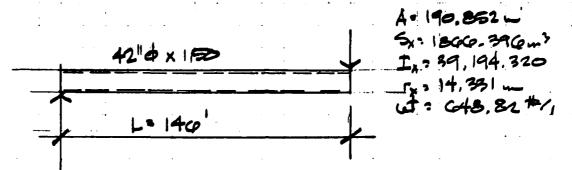
fotolo] = 27.7 kis.

fy total = 4=4k + 0,071 /m (804) . 2,03

folo = 27.7 + 2.03 = 29.73 kg < 30 kg = 0.k,

CREST OFFSHORE, INC. By LIMS __ CILIMS Sheet 1 10 of ____ By LIMS __ CILIMS Sheet 1 10 of ____ Date 9 (2 1 2 Job No. 27-771-94 _ Calculation ____

CHEEK MAXIMUM LEHOTH of PILE OF PICKUP



Max Max = w 22 = 0.049 1/(146)2 = 1729.31k

9.7 SOIL DAY - P-Y CURVES - STRUCTURE + 138 7675 14395 24790 3051 5089 20.00 20.00 34566 20.00 20.00 20.00 20.00 11794 24790 237 ğ y, DEFLECTION IN INCHES TYPICAL CURVES P-Y DATA 54 2857 1907 **E** 0.70 DIAMETER PIPE PILES 3 1122 0.41 0.4 0.41 POUNDS PER INCH P, SOIL RESISTANCE IN 1718 206 1135 150 2885 9794 876 4535 5579 2 0.25 0.24 0.24 0.24 0.24 0.25 0.27 42 - IN. 6794 123 185 35 <u>8</u> 88 8 E STRATIGRAPHY ASSUMED FOR PLY DATA 0.00 0.083 980.0 0.0 0.095 0.085 0.13 198 1669 \$ 0.053 0.013 90.0 0.063 0.063 0.019 0.00 0.00 0.013 0.031 0.037 0.075 Fine Sand and Silty Fine Sand Silty Fine Sand 0 0 0 0 0 0 Typical Cerve Inches 0 & 60 253 33 273 \$ 3 8 252 42 9 4 4 8 0 6 8 2 8

SECTION 10.0
INSTALLATION ANALYSES

10.1 INTRODUCTION

This section contains the analyses considered pertinent to the installation of the structure.

Section 10.2 includes the check of the stresses on the structural members at the mudline due to the soil pressure on the jacket before the piling can be attached to the jacket.

Section 10.3 is the analysis of the recommended lifting eyes for the jacket lift. For the analysis of the recommended lifting eyes for the superstructure, refer to Report No. 27-771-98.

Section 10.4 is the left analysis for the jacket. The computer output for the anlaysis is in Appendix B.8. For the lift analysis of the superstructure, refer to Report No. 27-771-98.

Section 10.5 contains the floatation analysis for the jacket.

Reference Drawings:

3016265 Jacket - Elevations

3016266 Jacket - Plan at El. (+) 12'-0"

3016267 Jacket - Plan at El. (-) 13'-0" & (-) 47'-0"

3016268 Jacket - Plan at El. (-) 81'-0"

3016272 Jacket - Lift Eye Details

OFFSHORE, INC.

By ______ Client () S. Navy ____ Subject 12-12/2 et & MUU Struct
Date _____ T. (e. 76 ___ Job No. __ 21-171-94 __ Calculation ______

10.2 SOIL PRESSUPE ON SPUCTURE

Weight of Jicket = 29Cp K Weight of Anodes = 11 K Weight of Pile String = 95 K

146 x 0,649 5%

402 K

Bengancy of Jacket in-place =-118.7k

HET WT = 283.3 K

Projected Area of Structural Members (13 of Han)

28 FT of 18 0 = 8 1 S.F. 28 FT of 10 24 0 = 25 S.F. 109 S.F.

18/4/0,000 10344x0345

Pessore on Structural Members

P= 283.3 = 0.866 /FT2 3 x 1095.F

Lord on 18 4 Member Sn: 117.05 0.866 4× 1.5 = 1.299 1/FT Lord on 1034 4 Member Sx = 29.912 0.866 44× 1075 = 0.776 1/FT.

fieck 18" ϕ Member - L=2857- $\frac{42}{2}$ x12-1'=25.82 $f_0 = \frac{\omega l^2}{125} = \frac{(1.299)(25.82')^2(12)}{12 \times 117.05} = 7.40 \text{ kg}$ 7.40 kg < 21.6 kg : 0, k.

Check 1034" Member - L: 28 - (24) = -26'

To: \omega \frac{12}{12} = \omega \frac{176}{2} \left(\frac{26}{2} \right)^2 = 17.54 km'

17.54 km < 21.6 km' : o.k.

CREST OFFSHORE, INC.

By Wa ___ CILON U.S. MAYT __ Subject PESION of BI MCW Streeture

Date _ 9.6. 7.6. Job No. _ 2] - 711 - 94 __ Calculation _______

10.3 LIATING EYES - LockeT

UKESI UFFSHUKE, INC.

Sheet 10,0561 _ _

By Lalbet Client U.S. Nary _ _ Subject Design at _ MLW Structure _ Date 8-17-16 Job No. _ 27-771 _ _ _ Calculation _ _ _ _ _ _ _ _ _ _ _

Lifting Eyes - Jacket

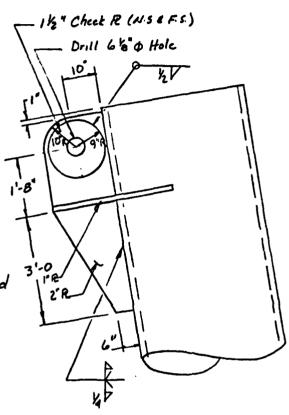
Vertical Lift

Weight of Jacket = 380K (excluding boot landing & bumpers)

Assumptions:

- 1. Enfire weight is at one lift eye.
- 2. Impact Factor of 2.0.
- 3. Total applied load can be acting completely horizontal or completely vertical.
- 4. Sling 0 = 60° for maximum load

.. Max P = 880 t at 0 = 60°



Check Shear in Pin

Use 6.00" Pin in double shear

$$f_s = P = \frac{8R0^k}{A} = 15.6 \text{ ksc}$$

Fs = 0.4 (36 ksi) x 1.33 = 19.2 ksi

15.6 < 19.2

By J. Talbat Client U.S. Nary _ Subject Design of _ MLW Structure _______
Date 8-17-76 Job No. 27-771-___ Calculation ______

Lifting Eyes - Jacket

Vertical Lift

Check Bearing on Plate

 $f_{br} = \frac{P}{Dt} = \frac{880^{K}}{(6.00)(5.00)} = 29.3 \text{ ksi}$

Fbr = 0.9 (36 ksi) = 32.4 ksi

29.3 < 32.4

Check Pin Shearing Through Plates

 $A = 9[(9-3)\times 1.5] + 2[(D-3)\times 2]$

A = 36 + 28 = 64 m2

fs = 880 = 13.8 Est

Fs = 0.4 (36 ksc) x1.33 = 19.2 ksi

13.8 < 19.2

Sheet 10_07ot _ _ _

By V. Talbot Client U.S. Nary _ subject Design of _ MLW Structure _ Date 8-17-76 Job No. 27-771- _ Calculation _ _ _ _

Lifting Eyes - Jacket

Vertical Lift

Check Tension Through Lift Eye

 $A = A[(9-3) \times 1.5] + 2[(10-3) \times 2.0]$

A - 64142

 $f_{t} = 880^{R} = 13.8 \text{ ksi}$

Ft = 0.6 (36 esi) x1.33 = 28.7 ks.

13.8 L Z8.7

Check Combined Bending and Tension

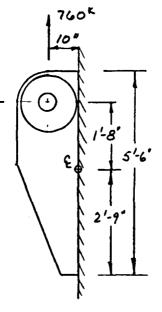
Shear Force = 760 t Tension Force = 760 (Conservative)

760K

Me = 760" x 20" - 760 " x 10" = 7600 in-k

$$f_s = \frac{760^k}{(66^l)(2^l)} = 5.8 \text{ ksi}$$

$$f_n = \frac{7600(6)}{2"(66")^2} + \frac{760"}{(66")(2")} = 11.0 \text{ ksc}$$



Using Mohr's Circle,

$$f_{n,max} = \frac{11.0}{2} + \left[\left(\frac{(1.0)^2}{2} \right)^2 + \left(5.8 \right)^2 \right]^{\frac{1}{2}} = 13.5 \text{ ksi}$$

Sheet 10.0001 _ _ _

By J_ Talkat client U.S Navy _ Subject Design of MUW Structure _ Date B-17.76 Job No. _ 27-771 = _ Calculation _ _ _ _ _

Lifting Eyes - Vacket

Vertical Lift

Assuming an average shear distribution,

Assume shear distribution parabolic, maximum shear at center of plate.

$$f_n = \frac{760}{(66')(2.0'')} = 5.8 \text{ ksi}$$

$$f_{smax} = \left[\left(\frac{5.8}{2} \right)^2 + \left(8.7 \right)^2 \right]^{1/2} = 9.2 \text{ Esc}$$

9.2 < 19.2

Check Weld of Check Plates

$$AR = 1.5 = 0.30$$

A Total 5.0

$$\frac{P}{C} = \frac{264^{K}}{\pi R^{"}} = 4.7^{K/m}$$

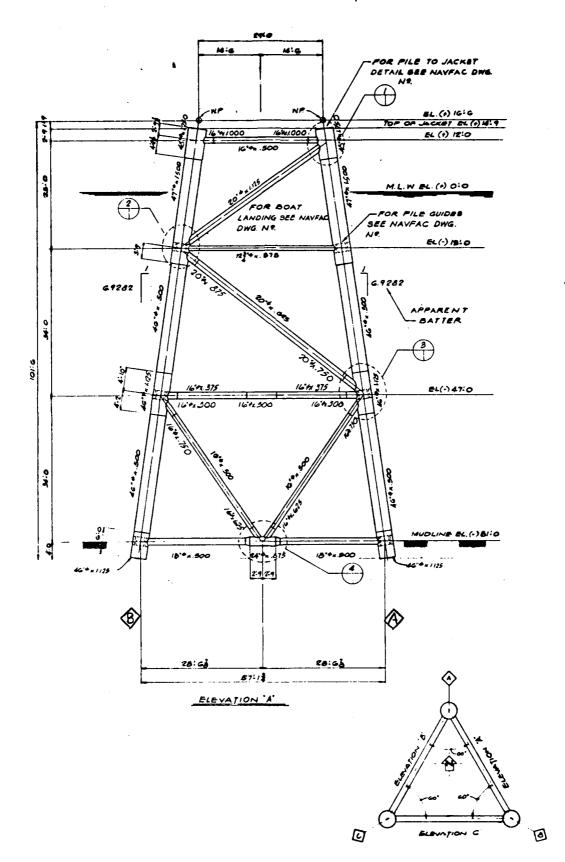
$$\omega = \frac{4.7}{11.2} = 0.12 \text{ in } Use \frac{1}{2}$$
 fillet kleld

10.4 LIFT AHALYSIS

(

Sheet 10,10 of _ _ _ _

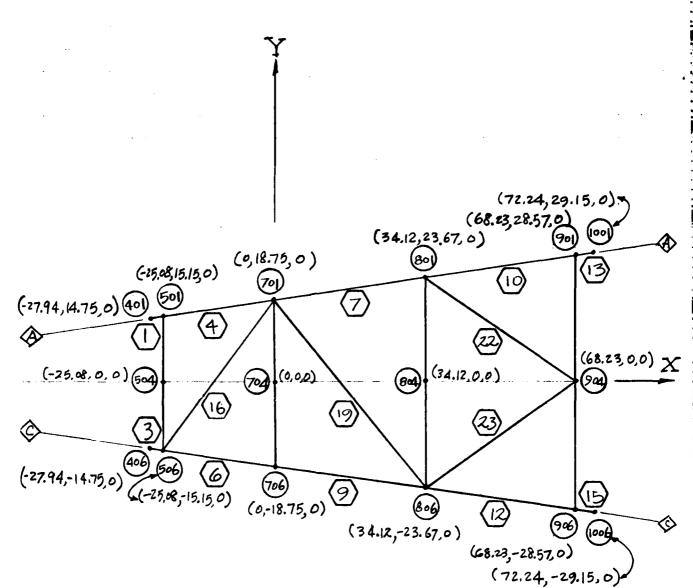
By C. Chern Client U.S. NAUY __ subject Lifting Analysis _____ Date 8-23-76 Job No. 27-771-01 _ calculation Platform #1 _ _ ___



CREST OFFSHORE, INC.	C	R	ES	T	0	F	F	S	H	0	R	Ε		INC.
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By C. Chern client U.S. NAVY _ subject Lifting Analysis _ ___ Date 8-20-76 Job No. 27-7 ZL-OL _ calculation Platform #1 _ _ _ _

Sheet 10.11 of ___

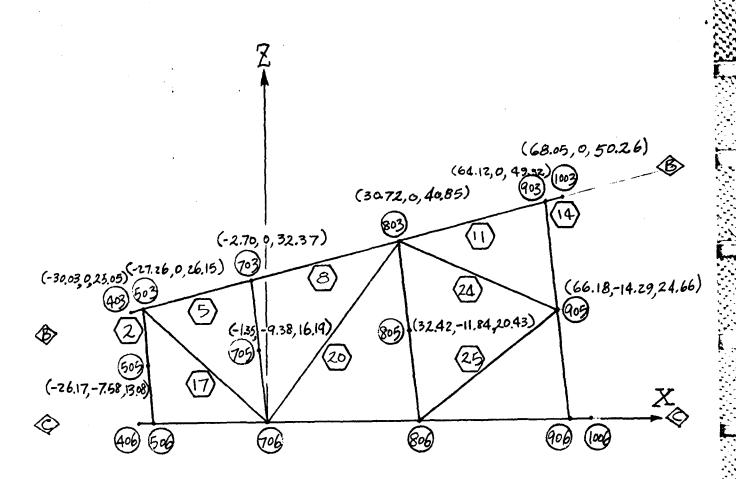


$$34 \times \frac{\sqrt{37}}{6} \times \frac{6.9282}{7} = 34.12$$

$$25 \times \frac{\sqrt{37}}{6} \times \frac{6.9282}{7} = 25.08$$

Sheet 1012 of ___

By C. Chern client U.S. NAVY _ subject Lifting Analysis _ ____
Date 8-20-76 Job No. 27-7-71-01 _ calculation Platform #1 _ _ _ ___



$$\frac{\text{Joint}}{503} = 26.24 \times \frac{12}{\sqrt{145}} = 26.15$$
$$26.24 \times \frac{1}{\sqrt{145}} = 2.18$$

$$\frac{403}{\sqrt{145}} = 25.05$$

$$25.14 \times \frac{12}{\sqrt{145}} = 25.05$$

$$25.14 \times \frac{1}{\sqrt{145}} = 2.09$$

$$\frac{301NT}{903} = 49.49x \frac{12}{\sqrt{145'}} = 49.32$$

$$49.49x \frac{1}{\sqrt{145'}} = 4.11$$

$$803 = 40.99x \frac{12}{\sqrt{145'}} = 40.85$$

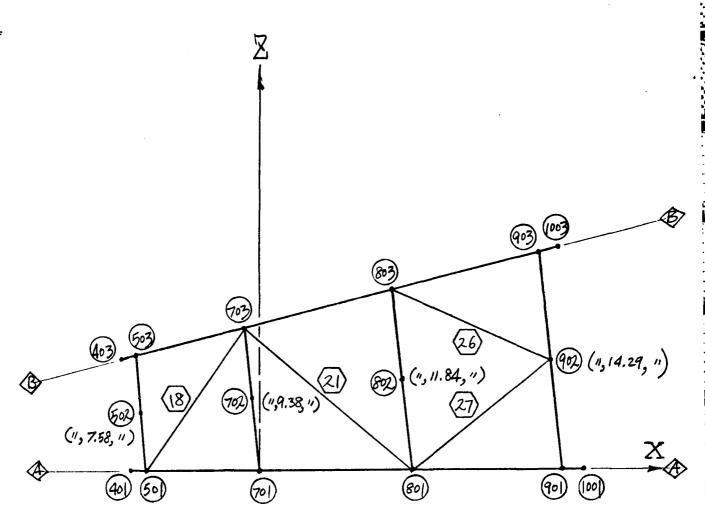
$$40.99x \frac{1}{\sqrt{145'}} = 3.40$$

$$\frac{703}{32.48} \times \frac{12}{\sqrt{145}} = 32.37$$

$$32.48 \times \frac{1}{\sqrt{145}} = 2.70$$

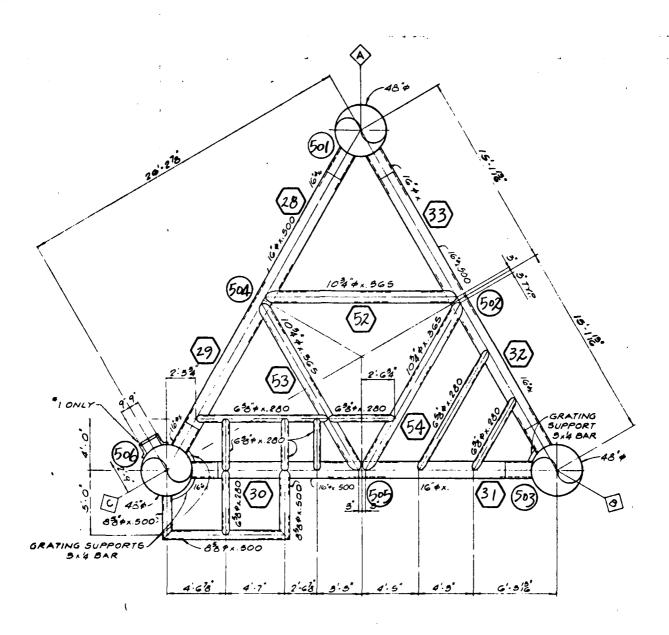
OFFSHORE, INC. **CREST**

Sheet 10. 1301 __ By C. Chern Client U.S. NAVY Date 8-20-76 Job No. 27-771-01 fting Analysis Platform 1__.



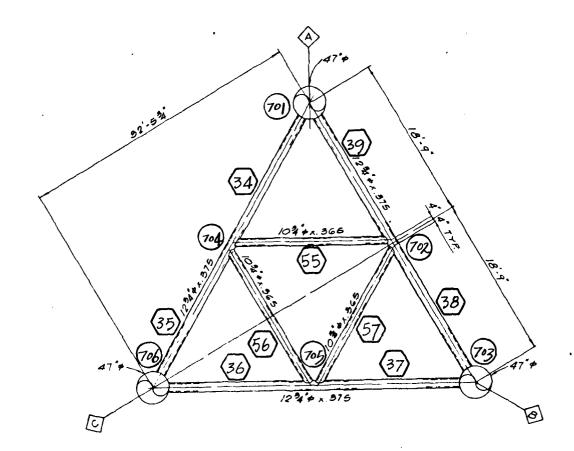
Sheet 10.14 of ____

By S. Chern Cilent U.S. NAUY __ subject Lifting Analysis _____ Date B=23-76 Job No. 27-771-01 _ calculation Platform#1 _____



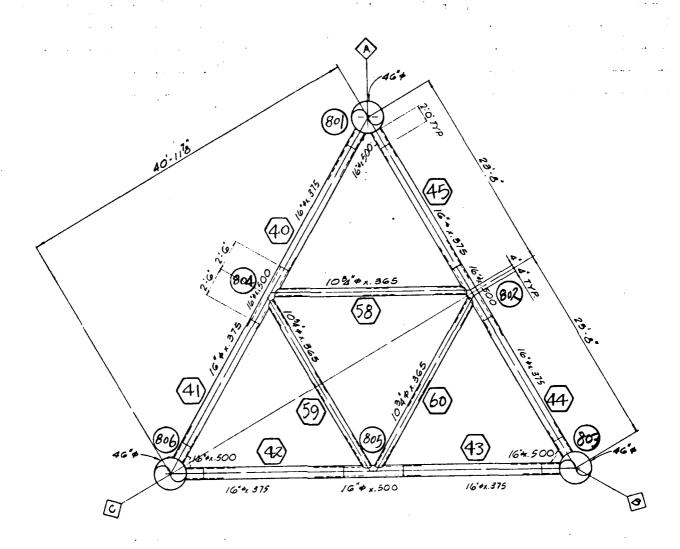
Sheet 10.15 of _

By C. Chern client U.S. NAVY __ subject Lifting Analysis ___
Date B-23-76 Job No. 27-771-01 _ calculation Platform #1 _ _ _

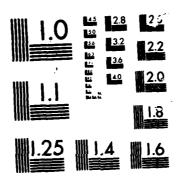


Sheet 10-1(0of ____

By C. Shern client U.S. NAUX _ subject Lifting Analysis _ ____
Date B=23=26 Job No. 27-771-0L _ calculation Platform #1 _ _ _ _ _



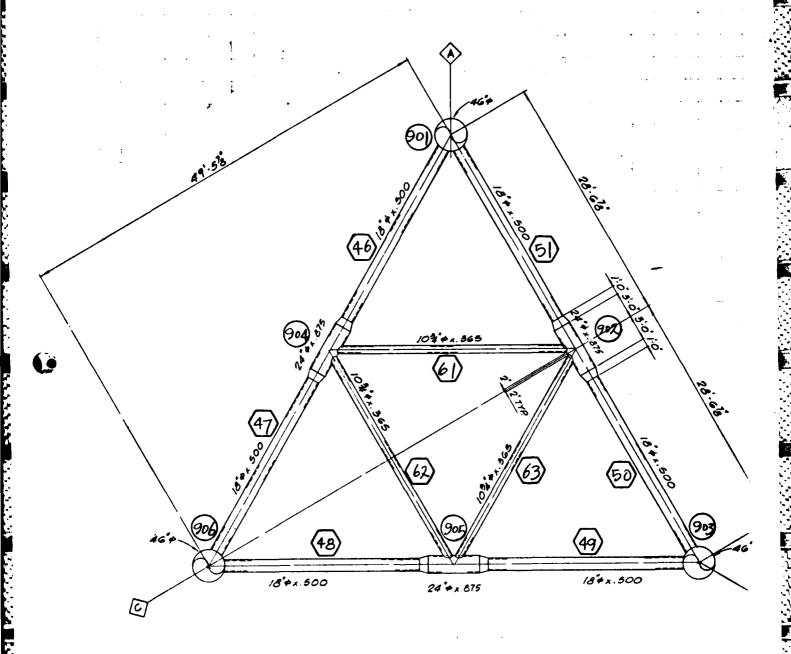
AD-A165 698 DESIGN CALCULATIONS 81' MLM STRUCTURE EAST COAST AIR COMBAT MANEUVERING R. (U) CREST ENGINEERING INC TULSA OK SEP 76 27-771-94 N62477-76-C-0179 3/8 UNCLASSIFIED F/G 13/13 NL



MICROCOPY RESOLUTION TEST CHART

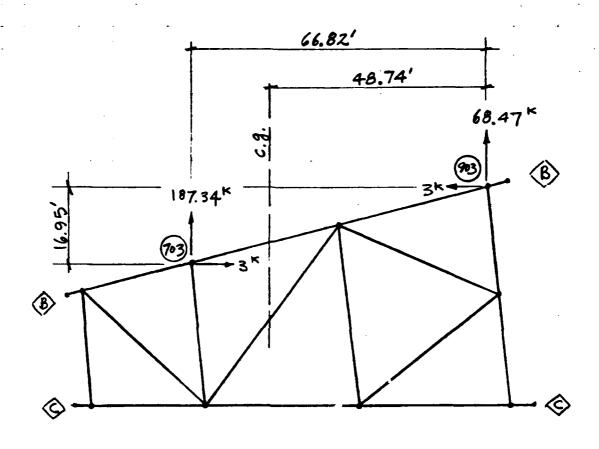
Sheet 1017 of ____

By C. Chern client U.S. NAUX __ subject_Lifting Analysis _____ Date 8-23-76 Job No. 27-771-01 _ Calculation Platform #1 _ _ ___



Sheet 10:18 of _ By C. Chern client U.S. NAVY _ subject Lifting Analysis_ Date B-25-76 Job No. 27-771-0L _ calculation Platform #1 _ _.

LOCATION OF CENTER OF GRAVITY



$$\bar{\chi} = \frac{187.34 \times 66.82 - 3 \times 16.95}{187.34 + 68.47}$$
$$= 48.74'$$
$$(48'-9")$$

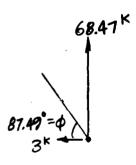
Sheet 10,19 of ____

By C. Chern CHAN L. S. NAVY _ subject Lifting Analysis _ _ _ _ Date 8-25-76 Job No. 27-77-01_ calculation Platform _ #1 _ _ _ _

JOINT NO. 703

$$\theta = \tan^{-1} \frac{187.34}{3} = 89.08^{\circ}$$

JOINT NO. 903



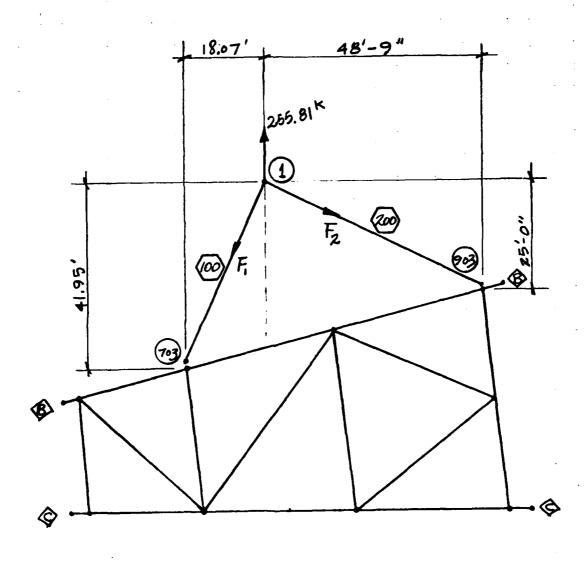
$$\phi = \tan^{-1} \frac{68.49}{3} = 87.49$$

(

Sheet 10,20 of ____

By C. Chern Cilent U.S. NAVY _ subject Lifting Analyzis _____
Date B=25-76 Job No. ZZ-771-01 _ Calculation Platform #1 _ ____

SINGLE POINT LIFTING (FOR CHECKING ONLY)



Sling Length *200
$$l_2 = \sqrt{25^2 + 48.75^2} = 54.79'$$

$$(54'-9'')$$
*100 $l_1 = \sqrt{18.07^2 + 4!.25^2} = 45.68'$

$$(45'-8'')$$

Sheet 10:21 of ____

By C. Chern Client U. S. NAVY _ subject Lifting Analysis _ ___.

Date B-25-76 Job No. 27-771-0 L _ calculation Platform #1 _ _ _ _

Equilibrium @ Px. 1

$$\left(F_{2} \times \frac{25}{54.79} + F_{1} \times \frac{41.95}{45.68} = 255.81 \right) \tag{9}$$

$$\frac{48.75}{54.79} F_2 = \frac{18.07}{45.68} F_1 \tag{b}$$

From Eq (b)
$$F_2 = \frac{18.07 \times 54.79}{45.68 \times 48.75} F_1 = 0.442 F_1$$

From Eq (a)
$$\left(\frac{18.07 \times 25}{45.68 \times 48.75} + \frac{41.95}{45.68}\right) F_1 = 255.81$$

$$F_2 = 0.442 \times 228.16$$

Sheet 10,22 of -By _ WA _ Client C.S. HAY _ _ Subject Design of & Mich Structs
Date _ 9. 6. 76 Job No. 21-171-94 _ _ Calculation _ _ _ _

10.5 FLOATATION AHALYSIS

Buoyancy of Jocket in Laurch Mode = 349.5th (Ref. Section 10.5, p. 10.23)

Weight of Jecket in Launch Mode

Material Listing : 296 K Anodes : 11 K

349.5 > 207k = Floatation is acheived

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By L. Kick _ Client U.S. NAVY _ Subject DESIGN OF BI'MLW STRUCTURE
Date 9-1-76 _ Job No. 27-771-94 _ Calculation BOUYANCY-LAUNCH _ _ _ _

DESCRIPTION	Q14.	VOLUME SEA WATER DISPLACED	
48.0"\$x 1.75 WT	21.00 FT	263.76 FT3	
47.5" x 1.50 WT	73.86 FT	908.48 Ft3	
47.0 " × 1.25 WT	45,00FT	541.80F73	
45.5 " x 0.50 WT	160.83FT	1815.77FT3	}
24.0"\$ x 0.875WT	16.SOFT	51.81 Ft3	
20.0" X 1.125WT	114.27 FT	249.11Ft3	
20.0" \$ x 0.875 WT	9.00FT	19.62 FT3	o
20.0" × 0.75 WT	11.25FT	24.53 Ft3	
20.0"\$ x 0.625WT	153.54FT	334.72 Ft3	06
18.0" × 0.50 WT	138.96FT	244.57 Ft3	' '
16.0" × 1.00 WT	14.00 FT	19.46 Ft3	70
16.0" × 0.75 WT	21.00 FT	29.19 Ft3	<u> </u>
16.0"\$ x 0.625WT	18.00 FT	25.01 Ft ³	\sim
16.0" \$ x 0.50 WT	441.96 FT	614.32 Ft3	4
12.75" \$ 0.315 WT	106.89 FT	94.70 Ft3	3
10.75" × 0.365 wt	246.75FT	155,45 FT3	$ \mathcal{A} $
8.625" x 0.50 WT	18.42FT	7.47 Ft3	3
6.625" \$x 0.280WT	54.75 FT	13.09 Ft3	عا
3.50" × 0.216	22.50 FT	1.49 Ft3	8
2.875" x 0.375	6.00 FT	0.27 Ft3	d
2.375" \$x 0.154	274.50FT	8,24 F73	a
2.00 STL. PLATE	45.8 FT2	7.65 Ft3	\mathcal{A}
1.50 STL. PLATE	17.69 572	2.21 Ft3	
1.00 STL. PLATE	45.27 F 72	3.16 Ft3	'
0.75 STL. PLATE	47.10Ft2	2.94F73	
0.625 STL. PLATE	37.68Ft2	<u> </u>	
0,50 STL. PLATE	52.50 Ft2		
0.375 STL. PLATE	4.13 FT2	0.13FT3	
0.250 STL. PLATE	1.70 FT2	0.04FT3	
GRATING	196.28Ft2	16.29 = 73	
ANGLE 4"x6" x 3/8"	24.00 FT	1.25 F73	
	TOTAL	5461.29 FT3	

(Bourgancy) Weight of Sea Water Displaced = 5461.29 x 64 = 349522.56#

·:.

By L. Kick _ Citent_U.S. NAVY _ Subject DESIGN OF 81' MLW STEUCTURE
Date 9-1-76 Job No. 27-771-94 _ Calculation BOUYANCY-IN HACE.

DESCRIPTION	Qty.	VOLUME SEA WATER DISPLACED (DRY I.D.)	1.) FLOODED I.D. OR 2.) ABOVE WATER LINE	
48.0"\$x 1.75 WT	21.00FT	263.76Ft ³	- 263.76 Ft3 (2.)	
47.5" x 1.50 wt	73.86 Ft	908.48 Ft3	- 908.48 Ft3(1.42.)	JACKET :
47.0 " × 1.25 WT	45.00FT	541.80F13	- 486,00F73 ((,)	Leas :
45.5 "\$x 0.50 WT	160.83FT	1815.77873	- 1736.96F73(1.)	
24.0"\$ x 0.875WT	16.50FT	51.81F73		•
20.0" X 1.125WT	114.27 FT	249.11 Ft3		
20.0"\$ x 0.875WT	9.00FT	(9,62 F73		
20.0" × 0.75 WT	11.25FT	24.53 FT3		
20.0" × 0.625 WT	153.54FT	334.72 Ft3		
18.0" × 0.50 WT	138.96Ft	244.57 Ft3		:
16.0"\$x 1.00WT	14.00 FT	19.46 Ft3	19.54F73 (2.)	
16.0" × 0.75 WT	21.00 FT	29.19 Ft3		
16.0"\$ x 0.625WT	18.00 FT	25.02 Ft3	}	}
16.0" \$ 0.50 WT	441.96 FT	614.32 FT3	- 111.46FT3(2.)	() (a) = 1;
12.75" \$x 0.315 WT	106.89 FT	94.70 Ft3		(+) 12'-0"
10.75" × 0.365 wT	246.75FT	(\$5.45 FT3		
8.625 N X O. SO WT	18.42FT	7.47 Ft3	7.47 Ft3 (2.)	
6.625" PX 0.280WT	54.75 FT	13.09 FT3	- 13.11F73 (2.)	
3.50" × 0.216	22.50 FT	1.49 Ft3		
2.875" \$x 0.375	6.00FT	0.27 Ft3		
2.375" Px 0.154	274.50FT	8.24 Ft3		
2.00 STL. PLATE	45.8 FT2	7.65 Ft3	7.65F73(2.)	LIFTING
1.50 STL. PLATE	17.69 672	2.21F73	2.21 Ft3 (2.)	Eves
1.00 STL. PLATE	45.27 F 72		3.76FT3 (2.)	
0.75 STL. PLATE	47.10FT2			
0.625 STL. PLATE	37.68Ft2	1		
0.50 STL. PLATE	52.50 Ft2	2.19Ft3		
0.375 STL. PLATE	4.13 FT2	· ·		
0.250 STL. PLATE	1.70 FT2	0.04 = 73		
GRATING	196.28F+2		- 16.29 Ft3 (2.)	
ANGLE 4"x6" x 3/8"	24.00FT		_ 1.25Ft3 (2.)	
	TOTALS	5461.29 FT	3605.44 (FT	3)

(Bourjaney) Weight of Sea Water Displaced = (5461.29 Ft3)-(3605.99 Ft3)
× 64 = 118739.20#

SECTION 11.0

CORROSION PROTECTION

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11.1 INTRODUCTION

The surface area of a marine structure is divided into three zones for corrosion protection consideration, the Submerged Zone, the Splash Zone, and the Atmospheric Zone.

The Submerged Zone is protected from corrosion by cathodic protection through the use of sacrificial anodes. The Splash Zone is protected by using one half inch thick extra material in excess of that needed for strength and then painted. The Atmosphere Zone is protected with paint.

11.2 DESIGN DATA

Zones for Corrosion Protection:

- 1. Submerged Zone El. (-) 4.0 feet to El. (-) 81.0 ft.
- 2. Splash Zone El. (+) 11.0 feet to El. (-) 4.0 feet.
- 3. Atmospheric Zone- El. (+) 75.0 feet to El. (+) 11.0 feet.

Current Requirements:

Current Density = 6 MA/ft^2 of surface in water 2 MA/ft^2 of surface in mud

Design Life:

N = 20 years

By L. KIRK Client U.S. MANY _ Subject DESIGN OF 81 MLW STENCTURE _ Date 7-4-76 _ Job No. 27-771-94 _ Calculation SURFACE AREA CALCULATION _ _ _

11.3 SUBMERCED ZONE

			SURFACE	(Ft2) Total
LOCATION	DESCRIPTION	QTY	AREA	SURFACE
(ELEV.)			(F12-	AREA
-4'-0' TO-13'-0"	47" \$x 12 4" S.L.	3	156.8	470.4
li 61 h	20" \$ 15'-0" D.B.	3_	_78.6	235.8
-131-0"	123/4" \$x 37'-6" H.B.	3	125.2	315.6
-13'-0" (PLAN)	123/44\$x181-6" H.B.	3	61.7	185.1
	20" \$x 53'-11" D.B.	3	282.5	847.5
u u u	46" \$x 301-3" S.L.	3	364.2	1092.6
-47'-0"(PLON)	16" \$ x 47 - 2" H.B.	3	198.1	594.3
11 (1 M	103/4" \$ x23'-3" H.B.	3	65.3	195.9
-47'-0" To-81'-0"	46"4x 381-0" J.L.	3	457.6	1372.8
11 tt 14	16" xx 411-4" D.B.	6	173.6	1041.5
-81'-0" (PLAN)	18" \$ x 25'-(1" H.B.	6	122.0	732.0
11 11 K	24" \$x 5 46" H.B.	3	34.5	103.5
11 11 14	103/4" x 281-9" H.B.	3	80.7	+242.1
	·			7489.1
PILING	42"\$ x2.00 WT x 220'-0"	3	2419.0	+7257.0
				14746.1
	,•.			
	,0.			
			,	
	•			

By Wid ____ Client O. S. M. Subject DESIGN Sheet Let of ____

By ____ Client O. S. M. Subject DESIGN Sheet Let of ____

Date 9:6:70 Job No. 21-171-94 __ Calculation _____

TOTAL CUPPENT PEQUIPEMENTS

I= GmA x 7489 A² + 2mA x 7257 A² I= 59.4 Amps

CAPACITY of ALLOY

Use Alluminum - Zmc - Mercury Alby

C= 1250 amp-brs

TOTAL WEIGHT of SACRIFICAL AHODES

W+ = I x N x8760 = 59.4 x 20 x 8160

Wr: 8325.5#

Using 725# Anode

N = 8325.5# 11.48

USE 12 - 725 # Anodes

11.4 SPLASH ZONE

The Splash Zone is protected by first using one half inch thick extra material in excess of that needed for strength, and then by applying paint to the structural members in the zone.

11.5 ATMOSPHERIC ZONE

The Atmospheric Zone is protected by paint. The surface area of the structure requiring paint is 8,500 square feet. The surface area calculations can be found in Report No. 37-771-98, Section 2.7, Paint Area.

SECTION 12.0
MATERIAL LIST AND WEIGHT

12.0 INTRODUCTION

This section includes a material listing and total weight of each major component of the structure including the superstructure, jacket, boat landing, boat fenders, and piling. The material listing in this section is a summary and includes only the total length and weight of each particular shape for each of the major components. A more detailed listing is found in the computer output in Appendix B.9.

CREST OFFSHORE, INC.

By ______ Client U.S.NAYY _____ Subject PENCH OF BL MCW STRUCTURE

Date _ 2 - 5 - 76 Job No. 21-1711-94 ___ Calculation _______

12.2 MATERIAL LISTING & WEIGHT - SUFER STRUCTURE

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1	WE I GW														!			
†																		
-	MATERIALS														;			
(5	¥		200	, no	86.	70.5		2		600		ļ					
	911	TAL METGHT		6574	15070	9158,79	1987	1237	200		1046.69	90,71						
	-771-01	TOTAL																
	27-77																	
•	TURE	H		000	 	139,68			000		26.67							
•	SUPERSTRUCTURE	TOTAL LENGT			2	7	7 0 0	. « «	121		NY.			}				
•	SUPER	TOTA																
	THE RY																	
	LS SUI	DIMENBION		1		5.50					000	•						
	TERIA	3410	:	××	×××	× × >	< × >	* * *	××		X X X							
	BILL OF MATERIALS SUMMARY U.S.NAVY ACMR PLATFOHMS	NOMINAL								APE	22.	1						
	BILL U.S	2	919	42.000 50.000	30.00 12.75	12.75	A. 6. 4.	200		E								
									! !									

		·	; Te
BILL OF MATERIALS SUMMARY U.S. NAVY ACHR PLATFORMS SUPERSTRUCTURE	RUCTURE 27-771-01	BILL OF MATERIALS & MEIGHT	
CO PP X P	250,67	-3176,90 3885,35	
CHANNELS			
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PLATE			
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SOUZE OCTO	76.86	126921	* \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\
125 TEICENESS	15.51	66	
GRATING 7.360 LBB PER SO-FI	276,10	-2032.10	
TOTAL MEIGHT		132752,69 188	
			12
			64

12.3 MATERIA CISTING & WEIGHT - LICKET

																						•
27-771-01 BILL OF MATERIALS & MEIGHT	TOTAL METOMT (POUND)			27510.57	45.38.98 45.08.08 80.08.08	28630,32	NO. 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			10049.62	e to a contract of the contrac	9000	49.00 P	24.071	1000							
1 FT MLW JACKET	TOTAL LENGTH (PEET)		21,00	60.64	160,83	110,27	11,25	183,54	0000	47.85	362,13	246,75	- E	22,50	274.50							
BILL GE MATERIALS SUMMARY U.S.MAVY ACMR PLATFORM BE	NOMINAL DIMENSION	1 d m	.000 n.D. x 1.750	000 0.0 X 1.250	500 0.0 x 0.500 000 0.0 x 0.475	.000_0.0.x_1.250	000 0.0 x 0.875	.000 D.D. X0.625	T 0000 K 10 C C C C C C C C C C C C C C C C C C	000 0.0 x 0.625	CO CO CO CO CO CO CO CO CO CO CO CO CO C	750 0.0 x 0.369	625 0.0 x 0.500	500 0.0 × 9.216	375 0.0 X 0.375	•						

71=01 BILL OF MATERIALS & WEIGHT		204,77		1340,440 1340,447	2000	1100 CO 1100 C	17,33		295490,75 L85			4	
FT MLW JACKET 27-77		60,48			2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	80 50 50 50 50 50 50 50 50 50 50 50 50 50	1.70	46, 40	MESCHT				
BILL OF-MATERIALS-SUMMARY U.S.WAVY ACMR PLATFORM SI F	ANGLE	4.000 x 6.000 x 0.375	1	a a a a	FFF		=	GRATING 7. tho I Ma BFP NO FT	נסגער				

CREST	OFFSHORE,	INC.	_	share)2/0/24
ву	Client_U_S_	7777. 	Subject Party 5	Sheet 1210301
Date	4 LO JOB NO. 27	-171-94_	. Calculation	

12.4 MARGIAL LISTING & WEIGHT - BOAT LANDING

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(FEET) (POUND)			3,33 16,74 65,00 316,12 75,00 204,03		2,50 51		3,00				
NOTENAL DIRECTOR	× ×	0.00 x 0.432	2375 0.0. X 0.218 HT 1,900 0.0. X 0.281 HT 1,900 0.0. X 0.145 HT	CHANNELS	C 12 x 20,70	ANGLE	2,000 X 2,000 X 0,250				

Carrier massacras carresses assesses assesses

12.5 MATERIAL LISTING & WEIGHT - BOAT FEMPERS

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٤	27-771-01 BILL OF MATERIALS & WEIGHT	TUTAL MEIGHT (POUND)		1515.05		2914.21 327.72		13425,25 1,83					
	BARGE FENDERS 27-7	TUTAL LENGTH (FEET)		40,75 12,37		54.00 95.16 16.05		2610X7					
�	U.S.NAVY ACHR PLATFURES	NOHINAL DIMENSION	Id T d	16,000 U,D, X 0,750 MT	PLATE	1,000 THICKPRUS 0,750 THICKNESS 0,500 THICKNESS		TOTAL					

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(

CREST	OFFSHORE,	INC.	Sheet 12.120f
By _JWS	Client O		Subject Tears of Si Man Shutter - Calculation
Date _ 1 6	2:16 Job No2	E 201-24 -	Calculation

12.6 MATERIAL LISTING & WEIGHT - PILING

CONTRACTOR COSTOS CALCASIA

U.S., AATERIALS SUMMARY				12. 14
JACKET PILING TOTAL LENGTH (PEET) 120,00 120	27-771-01 BILL TOTAL MEIGHT (POUND)	392541 (13 94876 (13 259120 (63 18821 (29 6033,24		
	JACKET PILING TOTAL LENGTH (PEET)	459,00 126,00 399,00 72,00 24,00		

APPENDIX A.1
ENVIRONMENTAL DATA

A.I ENVIRONMENTAL DATA

GENN'S WAVE DATA

TABLE 22:	50 YEAR STORM WIND, TIDE AND WAVE TICS: 35°57'N, 75°16'W: SPECI CHART DEPTH: OFFSHORE KITTY HAWK, CAROLINA	FIED 81 FOOT
	Chart Depth	81.0 Ft.
	Highest Astronomical Tide	4.5 Ft.
	Storm Tide	4.0 Ft.
	Total Tide	8.5 Ft.
	Still Water Depth	89.5 Ft.
	Height of Maximum Wave	60.3 Ft.
	Period of Maximum Wave	13.6 Sec.
	Crest Elevation of Maximum Wave Above Still Water Level	46.4 Ft.
•	Crest Elevation of Maximum Wave Above Chart Datum	54.9 Ft.
•	Crest Elevation of Maximum Wave Above Bottom	135.9 Ft.
	Length of Maximum Wave .	774.9 Ft.
	1 Hour Wind	114 Mph
•	0.5 Hour Wind	120 Mph
	I Minute Wind	145 Mph
	Maximum Instantaneous Gust	174 Mph

TABLE 23: 50 YEAR DESIGN CURRENT VERSUS PERCENT OF DEPTH: 35°57'N, 75°16'W: SPECIFIED 81 FOOT CHART DEPTH: OFFSHORE KITTY HAWK, NORTH CAROLINA

Percent Of Depth	Current Speed (Ft/Sec.)
0\$	4.3
10%	4.0
20%	3.6
30%	. 3.3
40%	3.0
50%	2.8
60%	2.5
70%	2.2
80\$	1.9
90%	1.6
100%	0.8

NOTES: ¹Recommended Design Current To Be Considered Simultaneous With 50 Year Wave... See Text.

> ²Use Drag Coefficient, C_D ≈ 0.68. For Combined Wave Current Veto-≿itles for 1.0 Ft. Diameter Pile. Use Conversion Factors in Table 21 For Other Diameters.

> > From July 7, 1975 Report

APPENDIX A.2

WAVE PROFILES

WAVE TROPILES

Dean's Wove for BI MALLY Structures with Tree tartore Effects Pressures at Chief and at +2019-

0-25-70

1.30 FT./SEC.. SURFACE CURRENT = HFIGHT # 60.30 FT. PFRIDD # 13.60 SECS., LENGTH WATER DEPTH # 89.50 FT.

3.00 FT., ROTTOM CURRENT =

CRESTIEL FT. DEPTH FIELD VARIABLES FOR WAVER

	-390.00	-360.00	-340.00	~ 4	-300.00	•	-260.00	-240.00	-220.00
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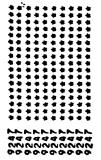
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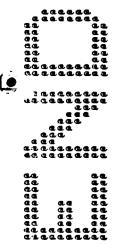
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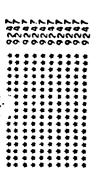
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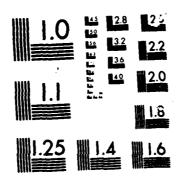
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CLARG   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   Superage   S	C	O A M M M M M M M M M M M M M M M M M M	
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Lian   Sud Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub   Sub		O A 3 LIND	
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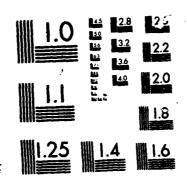
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MICROCOPY RESOLUTION TEST CHART

APPENDIX B.2

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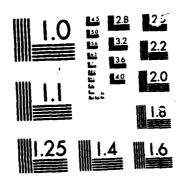
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1001/2   25924   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   1001/2   10	٠ تـ	1,56162	265	5.000.	300100	.00555	
	7	2.55.06	-1155"	<b>ラカーラン・</b>	- 00016	00534	
1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1,000,   1		5.14709	1070	2/2ng*-	00156	.00338	
1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,000   1,00	1/45/-1	\$505.	\$5455	5 0 0 0 4 5	9/000.	09500	
1,200   1,2000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,0000   1,	07.07.01	2		00307	00100	01500	
1	20117	•	. 35439	cc1 /5	00100	.00008	
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1,2554	-	•	1,54,0	20800	70100	00.539	
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56570		60057	44650	46400	.00159	.00617	
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### CACARA TERRES CRAMP TERRES

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46,55   1,55   1,55   1,55   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05   1,05		7	ני ניי	_ ર	د ع د ع	· .	~ :	3 : C	٦.	~ <	•		~;
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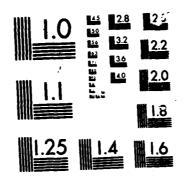
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AXIAL	4.725 4.010 6.634	1.092	351	7.108 7.108 7.108	6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1,197 1,092 6,248 4,854	0.0	₩ N N N N N N N N N N N N N N N N N N N	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
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-/ CALCULATED PUNCHING SHEAK		5,836		3,329	7.624		5.000 5.000 5.000	8,005		1,073	5,023		20.00	7.927	1	9 4 4 6	-	7.321	4,859		6,881	•		5,701		9,446	5,570
E 3 S =	5.927	6,272	8,516	2.068	6,116	8.241	3.05¢	7.186	5.509	•	90°C		4020	7.032	2000	679.7	7,582	5.617	6,145	4.061	6.072	961.9	605.4	4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4.561	7.000	
AXIAL	6.029	424	2.429	3,244	10.057	2,206	4,115	9,746	4,555	9699	4.167	. 10	25.24	9.780	7,619	7,612	2,654	7.304	2,778	6.456	6,522	3,541	6.467	0.00 0.00 0.00 0.00	A. 0.18	7,612	2,654
THICKNESS /	1.750	00	1.750	0000	1.000	1,750	000	1.000	1,750	1,000	1,000	1,750	o (	000	1,750	1.000	1,000	1.750	1.000	.75	1.000	1.000	1.750	0000		000	
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AXIAL		7.304	•			7,081		<b>.</b>	7. 8.48 8.48		2,801	S.	i.J	F.C.	3.537	20			4.011		30	2	•	1.571	• •	9.124	•	•	7 ° 7 ° 8
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SAPEHR - CREST OFFSHURE, INC. STRUCTURAL PUSTPHUCESSUR SYSTEM

Second Processing Supplicate Residence

PUNCHING	6.547 6.547	9,850	9.050	8,830	0.50.40	0,850	9,859	0,030	8,050	058.8	8,850	0508	8,850	0,050
PUNCHING	5,081 1,099	1,307	1,373	4,463	4,509	1,259.9	\$25.5	4.023	4,501	7,085	6,807	02929	4,867	7,315
BENUING	3,453 4,453 4,453	328.	2,985	3,551	3,465	3,073	3,697	3,392	3,468	5,592	3,689	3,618	3.0031	2000
AXIAL	8 . 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	2,242	2,005	,352 7,115	123	2,214	1,913	3,616	3,459	2.157	1,813	8,167	7,959	3,957
		.875	. 500	\$75	\$75	\$75	675 500	\$75	500	\$75	500	500	875 5500	500
	16,00	24.00	24.00	24,00	24,00	24,00	24,00	16.00	16.00	24.00	24,00	16200	16,00	24,00
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SAPCHK - CREST OFFSHURE, INC. STRUCTURAL PUSIPROCESSUR SYSTEM

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6.787	4,618	4,692	7,877	7,026	0,003	in 5	669.63	6,427	7.651	7,621
3.580	3,567	3,215	711	408.4	4,420	925	1,005	8.977	3,504	3.430
12,732	3,528	3,662	13,407	1.627	3.445	3,142	4 04U	12,469	3,370	3,640
000	875 500	500	5000	500	\$72	500	.500	500	\$75	. 500
10.00	24,00	24.00	24.00	24,00	24.00	24,00	100001	24,00	24,00	24,00
805 1004	316 1004	306 1004	803 1003	803 1003	803 1003	803 1003	806 1003	800 1005	606 1005	806 1005
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END UF RUN - DAPCHA

APPENDIX B.4

SAPACHK - Secondary Joints

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STHUCTURAL PUSTPHUCESS' FYSTEM THERE - CHEST OFFSHURE, INC.

. VAVY BIFT MLY STAUCTURE 27-771-01 PUNCHING SHEAR CHECK FUR STRAN

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4 6 3 5 3 5 5 6 PUNCHING SHEAR CHECK FUR STRAN ALLUMABLE PUNCHING SHEAN 9,326 9,326 4.326 9,326 9,326 9.326 4.320 9.326 9.326 9.320 8.654 6.659 8.614 8.814 ME B S . . CALCULATED
BENDING PUNCHING
SHEAM 2,402 3,120 3,116 2,279 2.092 1.895 2,775 3,985 2.270 2,397 2.481 STRUCTURAL PUSTPHOCESSUR SYSTEM 5.645 4.665 4.896 2.42 4.546 4.546 4.517 4.567 1.756 1.001 1.021 1.940 6.640 5.448 5.695 5.695 5.126 3.201 3.119 - BRAVY BIFT BLE STRUCTURE 27-771-01 10.605 4.852 710 1.603 754 1.705 368 2.108 6.504 2.106 1.625 2.191 11.454 715 2.201 1.501 .503 5.081 THICKNESS /= =S 5.263 1.500 2.2/1 365 242. 365 365 . 625 . 565 . 365 262. . 565 . 565 . 565 . 505 . 505 . 505 265 365 365 UIAMETER BAPCHA - CKEBT OFFBHUKE, INC. 10.75 10.75 10.75 10.00 10.75 10.75 10.75 10.75 10.00 10.75 PUSCHING SHEAK CHECK FUR BRACE 50 t 508 505 504 505 505 505 50S 50S 504 505 504 505 504 505 50**2** 502 502 502 505 505 205 206 502 502 502 50 c CASE JUINT 504 208 504 506 503 202 502 204 503 505 505 505 505 305 505 502 505 502 504 505 CHORU 501 203 501 105 105 201 105 505 503

JCHA - CHEST UFFSHURE, INC. STRUCTURAL PUSTPRUCESSI TYSIEM

POSCATING SHEAK CHECK FUR - LNAVY AIFT ALM STRUCTUME 27-771-01 PUNCAING SHEAK CHECK FUR STRAN.

CHEAR	6.913	8.915	;	200.0		084 4	9 8 8 9		9.880	NAB. B		8.850	8.650	;	9.880	9.880		388°S	0 0 0		8,880			3 5 3 6 3 6 3 6 3 6		A A A	9.680		8.830	088.0	
CHEAR	1.749	2,918	•	2,698	; ; •	000	2,691		3.052	2,578		.214	769.7	j	3.34	4.700		1,912	4,16		1.921	4.777		2 ° 4 ° 4 ° 4 ° 4 ° 4 ° 4 ° 4 ° 4 ° 4 °		475	5,049		2,094	86947	i
	5.550	4.461	5.488	4.067		000	2.715	5.739	3.050	4.095	0.470	222	4.713	6.087	0.570	4,765	4.786	1.707	4041	4.678	1,115	770.7	9.5.99	* * * * * * * * * * * * * * * * * * *	2 4 4	204	5.126	5,530	1.977	561.4	
	05%.0	1.665	8.844	1.504	,	6 D V 1	9.40	3.537	079.	289	3.611	. 043	1,137	3.832	500.	1,142	.542	. 60A	16516	1,222	1641	1,320	3,009	1,161	015.6		101-1	897.	049	1,316	
	\$50°	565	\$50.	565	200	245	365.	. 375	365	365	375	365	. 365	375	. 365	. 505	.375	. 365	2962	. 575	1,365	345	275	 	573	5	365	375	595	506.	
	16.00	10.75	16.00	10.75	11111		10.75	12.75	10.75	14.75	14.75	10.75	10,75	12,75	10.75	10,75	12,75	10,75	100/2	12,75	10.75	10,75	2.2	10.75	1	10.75	19.75	12,75	10,75	10,75	
	504	505		20.00	) )	1 3	705		704	205		704	705		0	705		704	2		704	70%		707		2	705		705	<b>o</b> 1	
	505	504	9	504	I	702	707		207	707		702	102		702	705		202	104		20.	<del>7</del> 0/	- 10	70¢		٦	707		702	0	
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PUNCHING SHEAR CHECK FUR ALLUWABLE PUNCHING SHEAR 0 2 2 2 7.914 7.914 7.914 7.914 7.914 7.914 7.914 6.880 0.880.0 7,914 CALCULATED PUNCHING SHEAN 3,630 1.026 3.661 3.611 2,123 2,144 3.613 2,396 2,468 2,555 3.642 1,691 STRUCTURAL PUSTPHUCESSUR SYSTEM THICKNESS /- -S T K E S G - -/ 5.508 6.035 4.040 2.721 3.527 3.558 5.410 5.661 1.590 4.433 4.674 4.43.4 4.73.2 2.65¢ 5.0¢¢ 3.05¢ 5.171 5.587 PURCHING SHEAR CHECK FUR - "NAVY HIFT MLR STRUCTURE 27=771=01 4.105 3.171 2.180 4,62/ 5.619 842 643 2,483 590 1.149 5,401 AAIAL .532 1,134 1,101 2.668 .821 .540 7.044 1.154 1.143 365 365 365 .375 .3^5 .365 365 500 505 202 265 265 565 265 .500 265 500 305 565 375 355 355 262 DIAMETER SAPCHK - CHEST UFFSHURE, INC. 10.75 10.00 10.00 10.75 10.00 10.00 12.75 16.75 10.75 10.75 10.75 10.00 10.75 10.75 10.75 BRACE 705 705 705 908 804 90g 80S 3 C B 8 CS 20 % 80 S 808 #05 805 805 804 704 808 200 **209** 40 B 80**2**00 80 & 80 & 702 CASE SUMBER 102 300 **↑** 000 **€**0 \$0**7** 208 808 200 200 705 208 802 705 705 832 802 **₹**0.**₽** 805 808 S C C CHURE NUMBER 202 805 703 103 700 803 803 805 108 801 **308** 

CHA - CHEST HPESHURE, INC. STRUCTURAL PUSTPRUCESSI . STEM

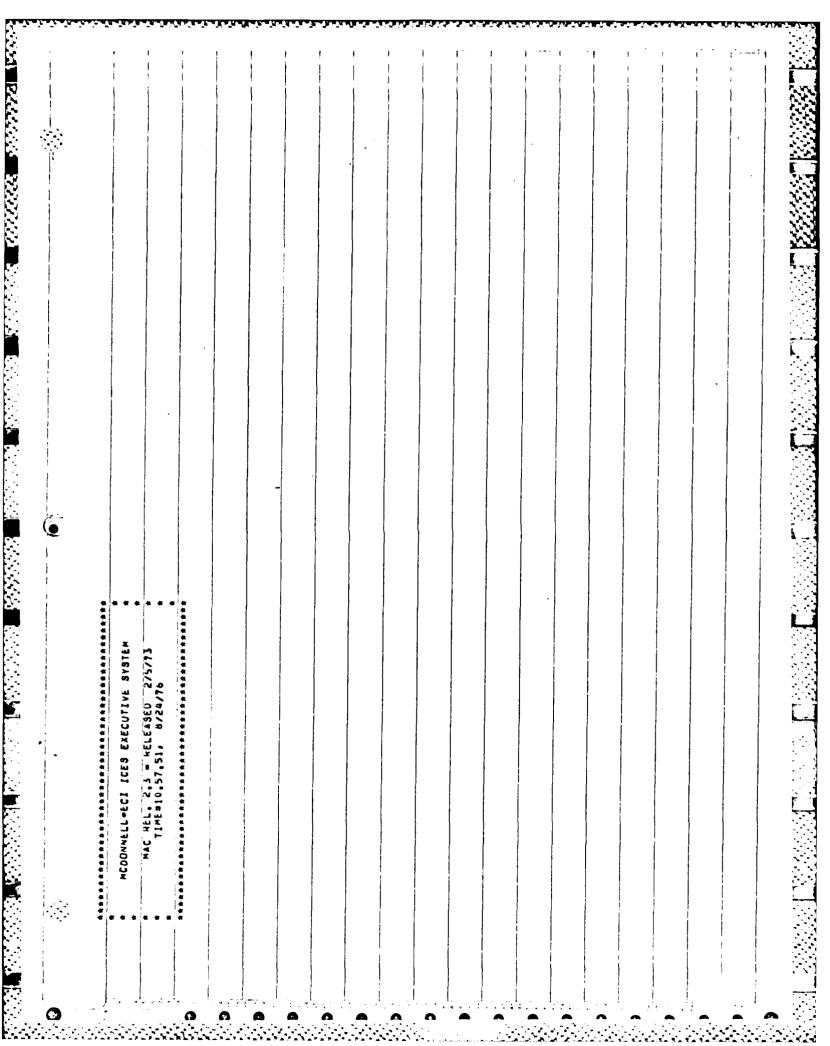
CALCULATED ALLU-ABLE PUNCHING BHEAN SHEAN SHEAN	2,167 7,914 4,196 7,914	Z.111 Z.914 d.121 7.914	1,211 7,914 4,326 7,914	1,101 7,914 4,339 7,914	2.014 6.733	Z.015 6.733	2,039 6,733	2.051 0.733	1,656 8,733	1,679 6,733	1,546 8,733	1,550 8,733	1,702 6,733
AXIAL HENDANG	5.507 2.430 .815 2.702 2	4,699 2,420 1,340 5,471 4	1.707 4.375 162 1.070 1	1,692 4,446 102 1,612 1	2,242 ,328 547 5,27/ 2	2,005 ,400	.352 .693 2.693 .5.633 .2	,123 ,680 ,045 5,860 2	2,214 ,219 517 4,851	1,913 ,339	3.616 .504 .956 3.545	3.459 ,052 963 ,556 1	2,157 ,692 ,535 4,591 1
Ularelek Talokaeu	10.10 ,500 10.75 ,565 11.75 ,565	10.00 .500 10.75 .305	10.75	10.00 .500 10.75 .565 10.75 .565	24,00 .875 10,75 .365	24.00 .875 10.75	24,00 ,875 10,75 ,565	24.00 .875 10.75 .365	24.00 .475	24.00 .875 10.75 .365	24,00 .875 10,75 .545	24.00 .875 10,75 .865	24.00 .875 10.75 .365
CASE DUMBER	7 802 804 404 AUS	208 408 805 805	208 308 208 308	10 402 804 804 805	1002 1004	1002 1004	1005 1004	10 1002 1004	1002 1005	1002 1005	1002 1005	1002 1005	1002 1004
CACKO COLAT	709 709 109	801 804 804	401 BO4 BO4	401 BOR 604	7001 1005 1005	001 1005 1005	1001 1005 1005	7001 2001 100	002 1003 1004	1002 1003 1002	7003 1005	002 1005 100C	1004 1004

STRUCTURAL PUSTPHUCESSUR SYSTEM SAPCHK - CKEST UFFSHURE, INC.

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ALLOWABLE PUNCHING	Z 4 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2	B.733	6.733	9.733	6.733	8.733	8,733	6.733	6.733	6.733	6,733	6.733	8.733	6.733	6,733		
CALCULATED PUNCHING	CHE AN	1,659	1,989	1,984	.865	947.	1.756	1,719	1,663	1,061	\$16.	956	710.	.639	1.045		
E S S/ BENDING		4.212	5.086	5.074	1.453	1.120	4.114	4,023	4.308	157.4	1,095	1.014	1.006	¥00°	1.179		
/= =8 T R AXIAL		1,613	.035	.023	3.957	3.9/1	3.528	3.602	2,043	1,627	3.445	3.142	4.045	4.058	3.370		
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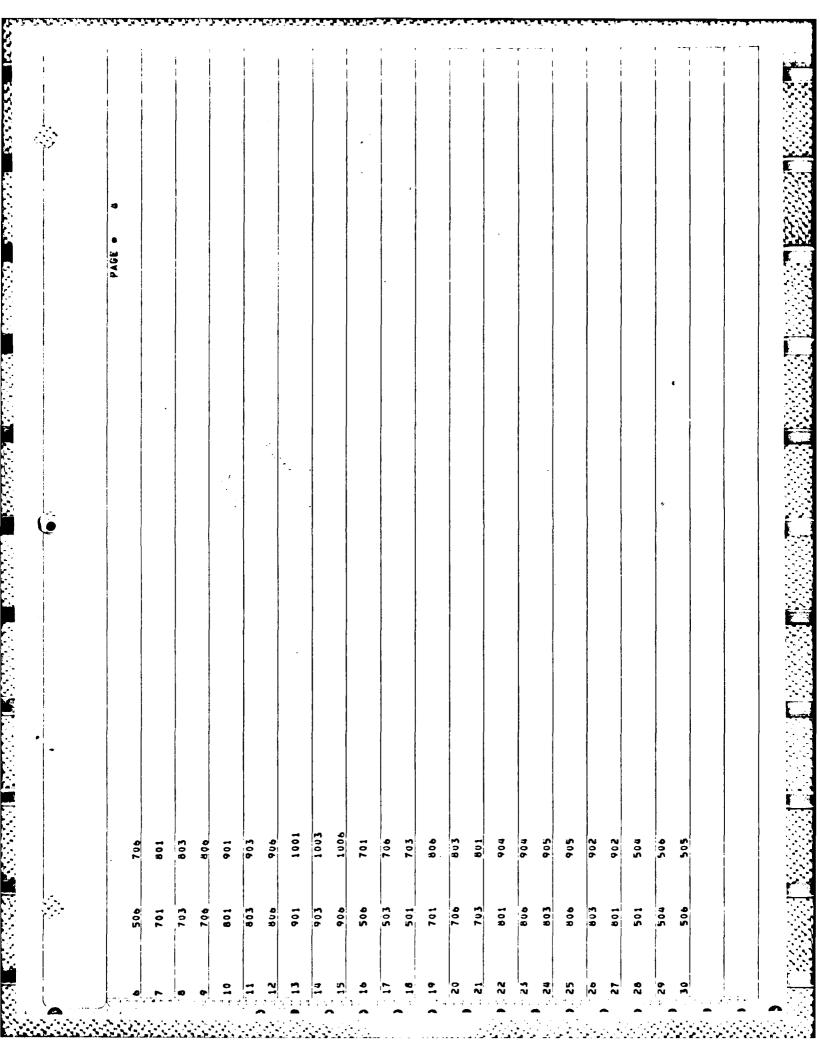
APPENDIX B.5

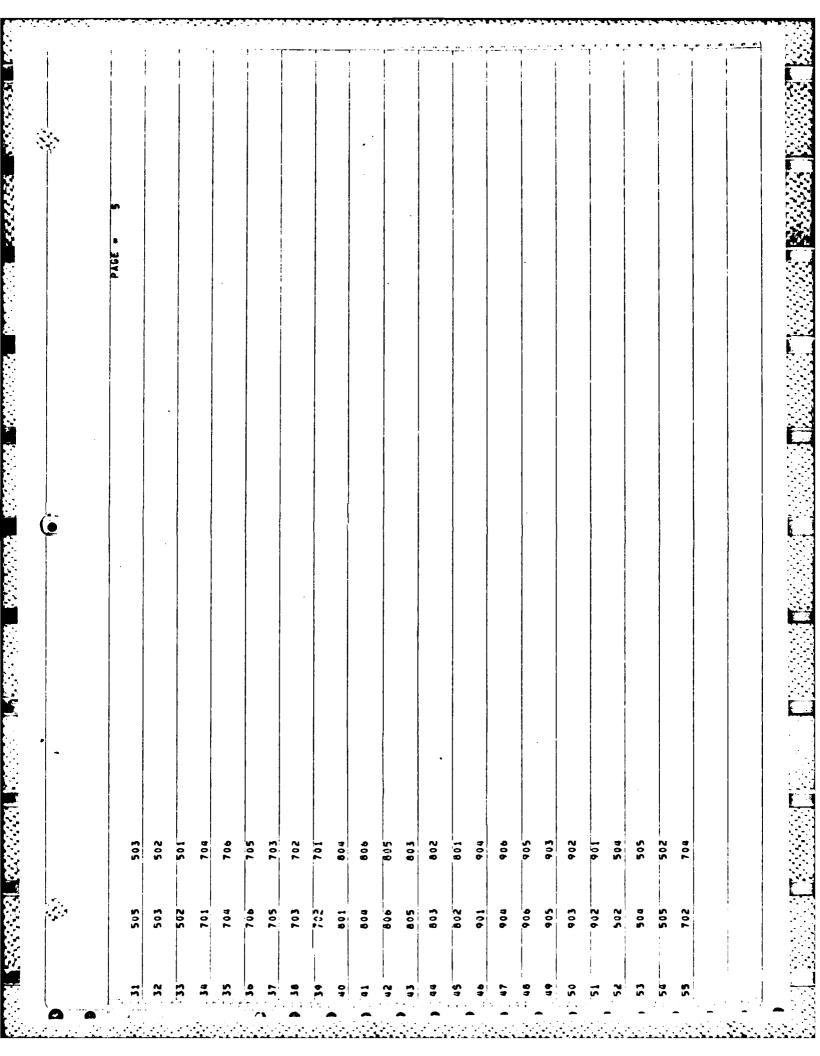
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7 TO 12 - AM 71,47 IX 36995,38 IV 18497,69	7,69 12 18497,69 SY 804,25 SZ 804,25	
13 TO 15 - AX 156.00 IX 79897,00 IY 39948,53	12 39948,53 SY	
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19 TO 21 - AX 38,04 IX 3574,08 IV	1767,04 12 1787,04 8Y 178,71 SZ 178,71	
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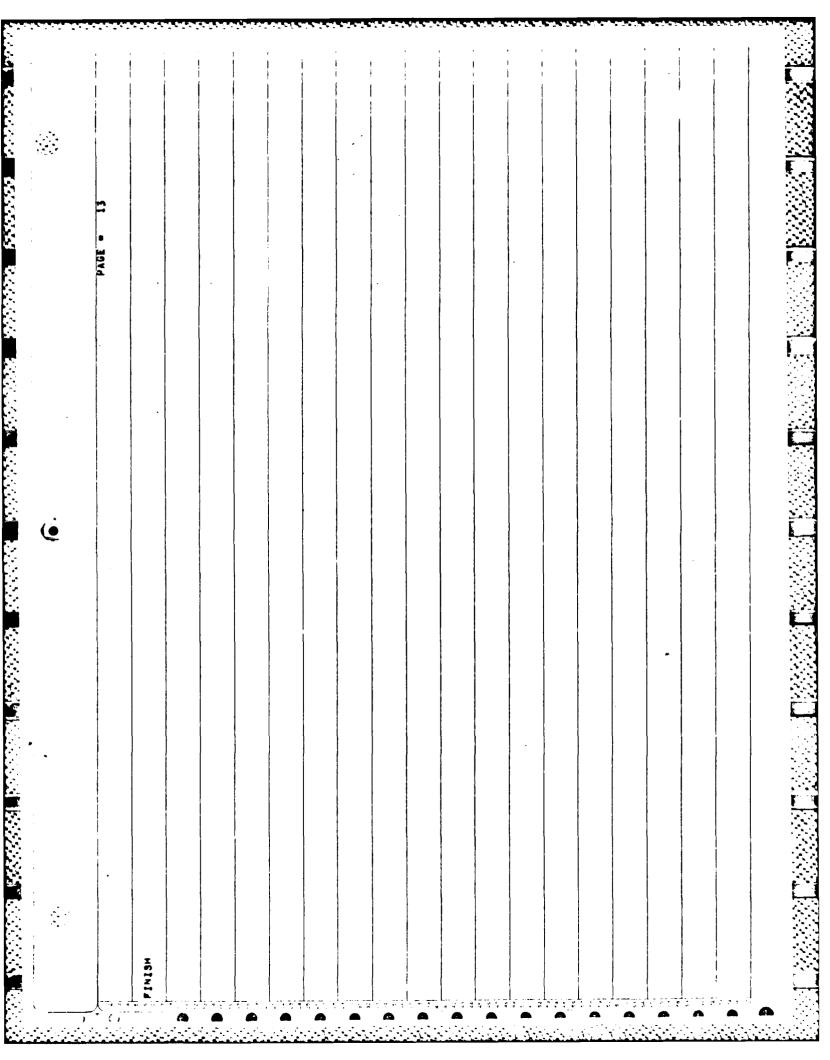
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7 TO 9 FOR 2 GLU UNI FR # #242,97 LA 0,11 LB 0,858	
7 10 9 FUR Z GLU UNI FR # -539,18 LA 0,858 LB 1.0	
. 10 TO 12 FUR Z GLU UNI FM -539,18 LA 0.0 LB 0.123	
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22 TO 33 FUR Z GLU UNI W -62,69	
34 TO 39 FUR Z GLO UNI W 449,52	
. 40 TO 45 FUR Z GLU UNI M -62,57	
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APPENDIX B.6
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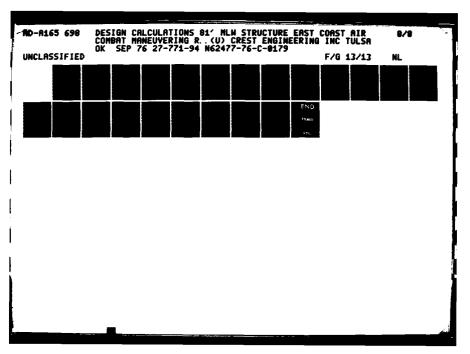
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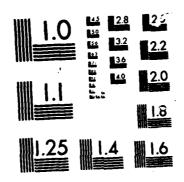
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